



## REPORT

# Annual Inspection

## *Stanton Station - Bottom Ash CCR Surface Impoundment*

Submitted to:

**Great River Energy**

2875 Third Street SW, Underwood, North Dakota 58576

Submitted by:

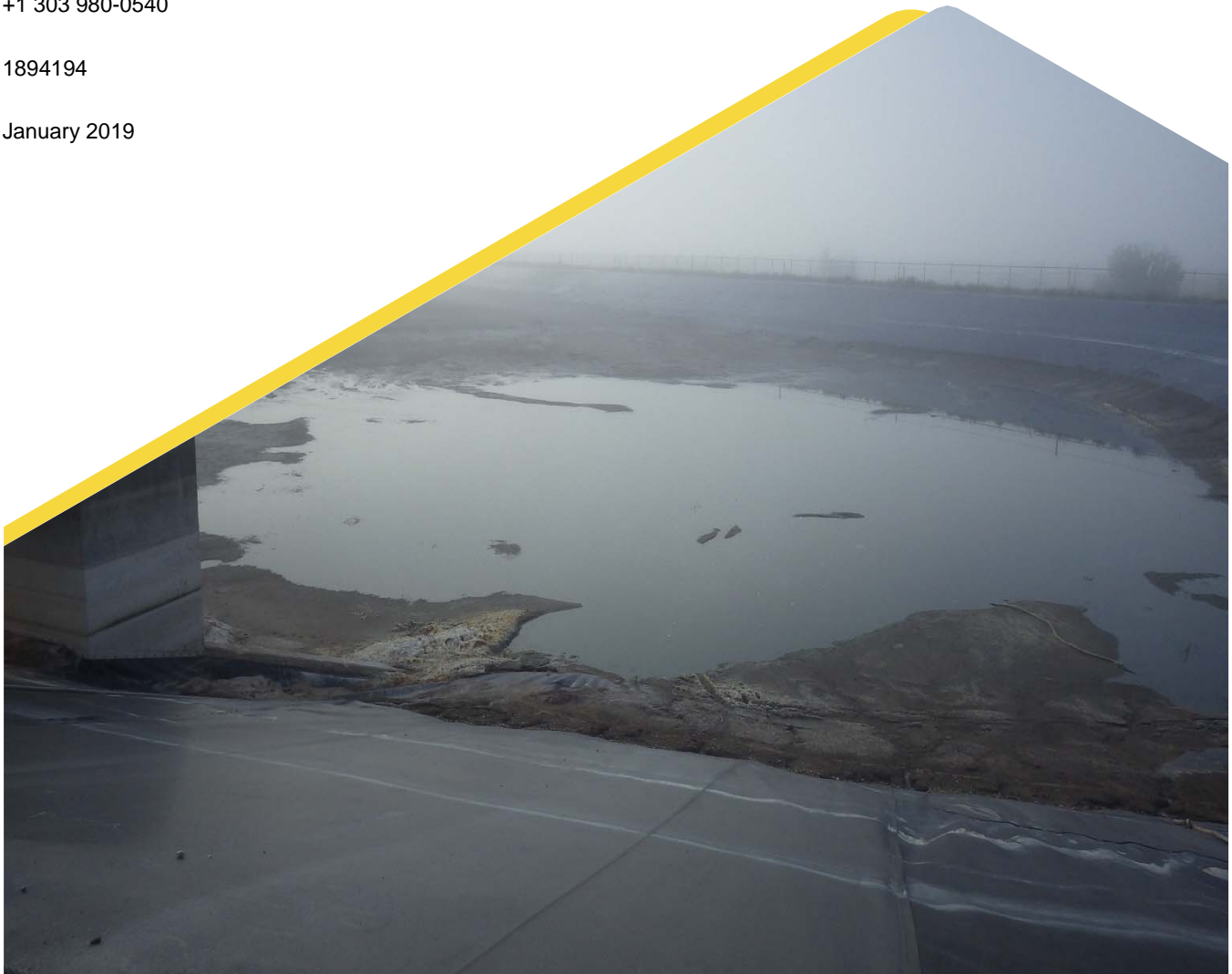
**Golder Associates Inc.**

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1894194

January 2019



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## 1.0 INTRODUCTION

As part of 40 CFR Part 257 of the Subtitle D solid waste provisions under the Resource Conservation and Recovery Act (RCRA), utilities are required to complete annual inspections for surface impoundments and landfills containing Coal Combustion Residuals (CCR). This report has been prepared by Golder Associates Inc. (Golder) for Great River Energy (GRE) to satisfy the annual inspection requirements for CCR surface impoundments under 40 CFR Part 257.83.

Stanton Station is located in Mercer County, approximately three miles southeast of Stanton, North Dakota. There are two facilities located at Stanton Station that fall under the CCR rule requirements (Figure 1). These facilities include the Bottom Ash CCR Landfill (Bottom Ash Landfill) and the Bottom Ash CCR Surface Impoundment (Bottom Ash Impoundment). Stanton Station ceased operation in February 2017 and these two facilities did not receive CCR materials in 2018. Plant decommissioning took place in 2017 with demolition occurring throughout 2018 and planned into 2019. The two facilities will remain open for disposal of any residual CCR in the plant, and construction and demolition debris associated with the plant deconstruction. This report presents a review of available facility information and findings of the inspection of the Bottom Ash Impoundment performed on September 18, 2018.

## 2.0 REVIEW OF EXISTING INFORMATION

### 2.1 Geological Conditions

Stanton Station is located in the Missouri Slope district of the glaciated Missouri Plateau of the Great Plains physiographic province (NDDH 2005). The Bottom Ash Impoundment is constructed in Missouri River alluvial deposits. The alluvial deposits have two distinct subunits: upper and lower. The upper subunit consists of a silty sand and clay and the lower subunit is an outwash sand and gravel (Barr 2010).

### 2.2 Site History and Liner Systems

Stanton Station is located in Section 16 and 21, Township 144N and Range 84W of Mercer County, three miles southeast of Stanton, North Dakota. The Bottom Ash Impoundment is divided into three cells, named the north, center, and south cells (see Figure 1 and Figure 2). The north and south cells were active cells used for dewatering bottom ash and the center cell functioned as a retention cell. Bottom ash was placed into one of the active cells until the cell reached capacity. Once capacity was reached bottom ash deposition was directed to the other active cell and the filled cell was dewatered. Bottom ash remaining in the dewatered active cell was excavated and hauled to the adjacent Bottom Ash Landfill for containment. Each active cell was sized to hold at least two years of plant bottom ash production (Stone & Webster 1994c).

Stanton Station originally burned North Dakota lignite before being converted in November 2004 to use fuel from the Powder River Basin in Wyoming. All ash was originally wet sluiced into a series of ash ponds (Ponds A, B, and C) (Stone & Webster 1994b). In the mid-1990s, Stanton Station converted to a dry fly ash handling system, and the historic CCR management units were reconfigured.

CCRs from the 1970s ash disposal area and Pond A were excavated and hauled to Ponds B and C for disposal. Ponds B and C were further consolidated and closed. Pond A was reconfigured to include a composite-lined surface impoundment with three cells and the Bottom Ash Landfill. The Bottom Ash Impoundment cells have floor liners consisting of two feet of protective cover, a 60 mil high density polyethylene (HDPE) geomembrane, and two feet of compacted clay fill (top to bottom). The liner along the side slopes consists of a 60-mil HDPE



geomembrane and approximately 3.2 feet of compacted clay (10 feet horizontal width). Selected construction drawings are included in Appendix A.

## 2.3 Site Geometry

The berm surrounding the Bottom Ash Impoundment and two interior berms have a top elevation of 1720 feet above mean sea level (amsl). The bottom elevation of the cells varies between 1700 feet amsl and 1704 feet amsl according to original construction drawings. The perimeter berm along the north, east, and south sides of the impoundment complex consists of a historic embankment to elevation 1715 feet amsl with a berm extension to 1720 feet amsl. The west perimeter berm and two interior berms were completely new construction. The berm extension and new berms were constructed in 1994 and 1995. The berm upstream and downstream slopes are 3:1. The crest is a gravel surfaced roadway supporting both light passenger vehicles and some heavy construction equipment.

## 2.4 Changes in Geometry

No significant recent changes to geometry were noted. The Bottom Ash Impoundment is planned to be closed in 2019 and 2020. The south cell will be closed with material remaining in place prior to construction of a final cover system that is in compliance with NDDH and CCR rule requirements. The north and center cells are planned to be closed by removal of CCR material, underlying liner systems, and applicable hydraulic structures.

## 2.5 Storage Capacity and Volumes

Up until February 2017, Stanton Station produced approximately 10,600 cubic yards (CY) of bottom ash and economizer ash (herein referred to as bottom ash) per year that was sluiced to the surface impoundment with water pumped from the Missouri River, demineralizer reject water, boiler blowdown water, water from the plant stormwater retention pond, the coal unloading pit sump, and from miscellaneous plant drains. The capacity of the Bottom Ash Impoundment cells to elevation 1720 feet amsl are:

- North Cell – 67,400 CY
- Center Cell – 59,400 CY
- South Cell – 75,600 CY

The north and center cells did not contain significant amounts of CCR at the time of the inspection. The south cell contained approximately 44,200 CY of bottom ash at the time of the inspection. Approximately 67,400 CY of bottom ash capacity remains in the north cell and 31,400 CY of capacity remains in the south cell. As previously discussed, the center cell does not directly receive CCRs and is not considered in the capacity calculations at this time.

## 2.6 Impounded Water

Water levels in the Bottom Ash Impoundment cells are controlled by stop logs at the inlet and outlet structures between the cells and at the discharge location. Only nominal amounts of water remained in the north and center cells of the Bottom Ash Impoundment at the time of inspection due to evaporation and discharge of that water via the permitted outfall. The water level in the south cell was at an elevation of approximately 1710 feet amsl. The approximate depths and volumes of impounded water for each cell during 2018 are shown in Table 1.

**Table 1: Depth and Volume of Impounded Water at the Bottom Ash Impoundment**

Cell Description	Minimum Depth of Water in 2018	Maximum Depth of Water in 2018	Depth of Water at Time of Inspection	Volume of Water at Time of Inspection	
	feet	feet	feet	gallons	acre-feet
North Cell	0	0	0	0	0
Center Cell	0	2	0	0	0
South Cell	0	3	2	550,000	1.7

## 2.7 Permits

The Bottom Ash Impoundment is currently permitted with the North Dakota Department of Health (NDDH) under Permit Number 0043.

## 2.8 Summary of 2018 Weekly Inspections

Routine weekly inspections of the Bottom Ash Impoundment were completed by GRE throughout 2018 as a part of the final CCR Rule. Based on a review of the available inspection forms, the following items were noted:

- Generally good site maintenance.
- No signs of significant seepage, settlement, or cracking of the berm downstream slopes.

## 2.9 Summary of Previous Inspections

The most recent annual professional engineer inspection of the Bottom Ash Impoundment was performed by Golder in September of 2017 (Golder 2018) and a summary of the observations of that inspection are as follows:

- Generally good vegetation and site maintenance of berm downstream slopes:
- Minor erosion and/or lack of robust grass vegetation.
- Animal burrows.
- Berm upstream slopes were in generally fair condition due to minor movement of soils underlying the geomembrane liner. Between 2012 and 2015, GRE performed repairs to the geomembrane and clay liner systems.
- Generally good condition of embankment crests, including the access roads.
- No signs of significant seepage, settlement, or cracking of the berm downstream slopes.

A previous “Coal Ash Impoundment Site Assessment Report” performed by Kleinfelder in 2011 (Kleinfelder 2012) under contract with the United States Environmental Protection Agency (USEPA) assigned the facility a “Less than Low” hazard rating and had similar observations with respect to facility stability.

## 3.0 2018 ANNUAL INSPECTION

On September 18, 2018, Todd Stong, Craig Schuettpelz, and Paul Schlicht of Golder performed an inspection of the Bottom Ash Impoundment per USEPA Regulation 40 CFR Part 257.83(b) requirements. The inspection consisted of visual observations while walking around the facility traversing up and down the perimeter berm. An annual inspection checklist used during the inspection is presented in Appendix B. Photographs were taken and are presented in Appendix C. The following presents a summary of the observations made during the 2018 annual inspection.

### 3.1 Hydraulic Structures

#### 3.1.1 North Cell

Inflow to the north cell includes the retention pond inlet pipe and the coal pit sump inlet pipe. The bottom ash piping was removed in early 2018 as part of plant deconstruction. The retention pond inlet and coal pit sump inlet pipes discharged into the impoundment onto a sacrificial HDPE wear-liner and were in good condition with no indications of wear or penetration of the liner or cracking of the inlet pipes.

Outflow from the north cell is through the concrete outflow structure located on the south berm upstream slope. The structure was in good condition with no signs of corrosion, erosion, or cracking. Sediment has accumulated within the concrete structure and pipeline connecting the north and center cells since these cells have been out of use since 2017. Stop logs were placed in the outfall structure to control the cell water elevation.

#### 3.1.2 Center Cell

Inflow to the center cell is through the outflow structures from the north cell and the south cell located on the north and south berm upstream slopes respectively. These structures were in good condition with no signs of corrosion, erosion, or cracking. Sediment has accumulated within the concrete structures and pipelines connecting the north and south cells to the center cell since these cells have been out of use since 2017.

Outflow from the center cell is through the outfall structure located on the east berm upstream slope. The structure was in good condition with no signs of blockage, corrosion, erosion, or cracking.

#### 3.1.3 South Cell

Inflow to the south cell includes the retention pond inlet pipe and the coal pit sump inlet pipe. The bottom ash piping was removed in early 2018 as part of plant deconstruction. The retention pond inlet and coal pit sump inlet pipes discharge into the impoundment onto a sacrificial HDPE wear-liner. The retention pond inlet and coal pit sump inlet pipes appeared to be in good condition.

Outflow from the south cell is through the concrete outflow structure located on the north berm upstream slope. The structure was in good condition with no signs of corrosion, erosion, or cracking. Sediment has accumulated within the concrete structure and pipeline connecting the north and center cells since these cells have been out of use since 2017. Stop logs were placed in the outfall structure to control the cell water elevation.

### 3.2 Berm Upstream Slope

#### 3.2.1 North Cell

Berm upstream slopes appeared to match the design slopes of 3:1 with no observed sections of significant slope difference. The geomembrane liner is exposed on the slopes with no protective cover. There were no signs of vegetation or animal burrows on the berm upstream slopes. Water was observed between the geomembrane liner

and compacted clay liner along the toe of the berm upstream slope in a couple of locations. Several geomembrane defects were observed on the upper portion of the geomembrane on the upstream slopes; however, the defect locations were observed well above the water elevation within the north cell observed over the course of 2018. The north cell berm upstream slopes appear to be in fair condition.

### 3.2.2 Center Cell

Berm upstream slopes appeared to match the design slopes of 3:1 to about 4 feet below the crest elevation (approximate elevation 1716 feet amsl) at which point the slopes curved inward to a steeper concave shape. This displacement of the lower slope is likely due to “pumping” of saturated soils beneath the geomembrane due to wave action and the lack of confining pressure from overlying protective cover. Water was observed between the geomembrane liner and compacted clay liner along the toe of the berm upstream slope and along the floor of the facility that was able to be evaluated; however, no signs of geomembrane defects were noted. The center cell berm upstream slopes appear to be in fair condition.

### 3.2.3 South Cell

The berm upstream slopes above the water level and not covered with bottom ash were evaluated. Berm upstream slopes appeared to match the design slopes of 3:1 with minor sections of slope difference where the slopes curved inward to a steeper concave shape. This displacement of the lower slope is likely due to “pumping” of saturated soils beneath the geomembrane due to wave action and the lack of confining pressure from overlying protective cover layer. The south cell berm upstream slopes appear to be in fair condition.

## 3.3 Berm Crest

The berm crest around the Bottom Ash Impoundment is surfaced with gravel at a constant elevation of 1720 feet amsl. The crest roadway is primarily used for light vehicle traffic, but is exposed to heavy construction equipment when the north and south cells are cleaned out. The berm crest appears to be in good condition with minimal weedy vegetation, no animal burrows, and no settlement. There was minor rutting along the berm crest road, but the rutting is limited to small areas and was most likely caused by light vehicle traffic during wet conditions.

## 3.4 Berm Downstream Slope

### 3.4.1 North Cell

The berm downstream slopes on the north and east sides are approximately 20 feet high and the berm downstream slope on the west side is approximately 5 feet high. The slopes are graded at approximately 3:1 and are mostly well vegetated with grass, with a few small areas of bare ground along the east side of the impoundment and the north side of the north cell that are susceptible to erosion. Small to medium size animal burrows were observed on the north and east berm downstream slopes. Berm downstream slopes are generally in good condition.

### 3.4.2 Center Cell

The berm downstream slope on the east side is approximately 20 feet high and the downstream slope on the west side is approximately 5 feet high. The slopes are graded at approximately 3:1 and are well vegetated with grass, with a few small areas of bare ground along the east side of the impoundment that are susceptible to erosion. Small animal burrows were observed on the berm downstream slopes. Berm downstream slopes are generally in good condition.

### 3.4.3 South Cell

The berm downstream slope on the east and south side is approximately 20 feet high and the berm downstream slope on the west side is approximately 5 feet high. The slopes are graded at approximately 3:1 and are well vegetated with grass, with a few small areas of bare ground along the east and south sides of the impoundment that are susceptible to erosion. Several small (2-inch diameter) animal burrows were observed on the east berm downstream slopes and several large-sized (6 to 10-inch diameter) animal burrows were observed on the south berm downstream slopes. The perimeter security fence on the south berm downstream slope near the southeast corner was also damaged. Berm downstream slopes are generally in fair condition due to the large number and size of animal burrows noted during the inspection.

## 3.5 Toe

### 3.5.1 North Cell

The toe of the west berm downstream slope is in the bottom ash deposition area and has no observed seepage, standing water, animal burrows, settlement, or excessive vegetation. The toes of the north and east berm downstream slopes have no observed seepage, standing water, animal burrows, settlement, or excessive vegetation. The toe of the berm downstream slopes around the north cell is in good condition.

### 3.5.2 Center Cell

The toe of the west berm downstream slope is in the bottom ash deposition area and has no observed seepage, standing water, animal burrows, settlement, or excessive vegetation. The toe of the east berm downstream slope is in a low area that has some marshy vegetation, but no standing water. There were no observed indications of seepage, animal burrows, settlement, or excessive vegetation. The toe of the berm downstream slopes around the center cell is in good condition.

### 3.5.3 South Cell

The toe of the west berm downstream slope is in the bottom ash deposition area and has no observed seepage, standing water, animal burrows, settlement, or excessive vegetation. The toe of the east berm downstream slope is in a low area that has some woody and/or marshy vegetation, but no standing water. There were no observed indications of seepage, animal burrows, settlement, or excessive vegetation. The toe of the south berm downstream slope is in a surface water drainage ditch that has some marshy vegetation, but no standing water at the time of inspection. There were a few observed small animal burrows during the inspection. There were no observed indications of seepage, settlement, or excessive vegetation.

## 3.6 Instrumentation

The Bottom Ash Impoundment currently has two piezometers (P-1 and P-2) on the berm downstream slope on the east side of the center cell (Figure 2). Water elevations observed in piezometers P-1 and P-2 were measured periodically in 2017; however, these piezometers were not measured in 2018 since the center cell of the Bottom ash Impoundment did not contain more than approximately 2 feet of water. The piezometers were evaluated regularly to determine whether they appeared to be in working order. In 2017, water elevations in both piezometers varied by less than approximately two feet over the course of the year and had average elevations of approximately 1693.4 feet amsl. These piezometers will be removed/abandoned as part of closure of the Bottom Ash Impoundment in 2019 or 2020.

### 3.7 Signs of Structural Weakness or Other Observations That Could Affect Stability

No signs of structural weakness or other observations that could affect the stability of the Bottom Ash Impoundment were observed during the site inspection in September 2018.

## 4.0 SUMMARY AND CONCLUSIONS

An annual inspection was performed for the Bottom Ash Impoundment at Stanton Station on September 18, 2018. The inspection met the requirements for CCR surface impoundments under 40 CFR Part 257.83. Golder observed good vegetation and site maintenance and did not identify significant deficiencies such as seepage, excessive erosion or settlement, or cracking during visual observations.

The Bottom Ash Impoundment is planned to be closed in 2019 and 2020. The south cell will be closed with material remaining in place prior to construction of a final cover system that is in compliance with NDDH and CCR rule requirements. The north and center cells are planned to be closed by removal of CCR material, underlying liner systems, and applicable hydraulic structures.

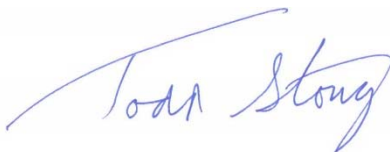
In addition to annual inspections of applicable portions of the facility by a Professional Engineer, trained and qualified site personnel will continue to perform the required weekly facility inspections to look for signs of potential structural weaknesses. Once the north and center cells are closed by removal of CCR, the south cell will be the only portion of the facility evaluated as a part of the CCR rule.

Minor maintenance items that may need to be continually addressed include repairing larger animal burrows as they appear, monitoring vegetative success of berm downstream slopes, removal of any woody vegetation growing on the berm downstream slopes, and repair of the perimeter security fence.

#### Golder Associates Inc.



Craig Schuettpelz, PE  
Senior Project Engineer



Todd Stong, PE  
Associate and Senior Consultant

KAC/CCS/ds

## 5.0 REFERENCES

Barr, 2010. 2010 Annual Groundwater Monitoring Report, Stanton Station Ash Disposal Facility, NDDH Solid Waste Permit # SP043. Prepared for Great River Energy, February 2011.

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Stone & Webster, 1994b. Stanton Station Ash Pond Modifications, Project No. 4177 Design Drawings Rev. 2. Prepared for United Power Association, June 1994.

Stone & Webster, 1994c. Plan of Operations Stanton Station Bottom Ash Surface Impoundment and Bottom Ash Landfill. Prepared for United Power Association, Project No. 4177. June 1994.

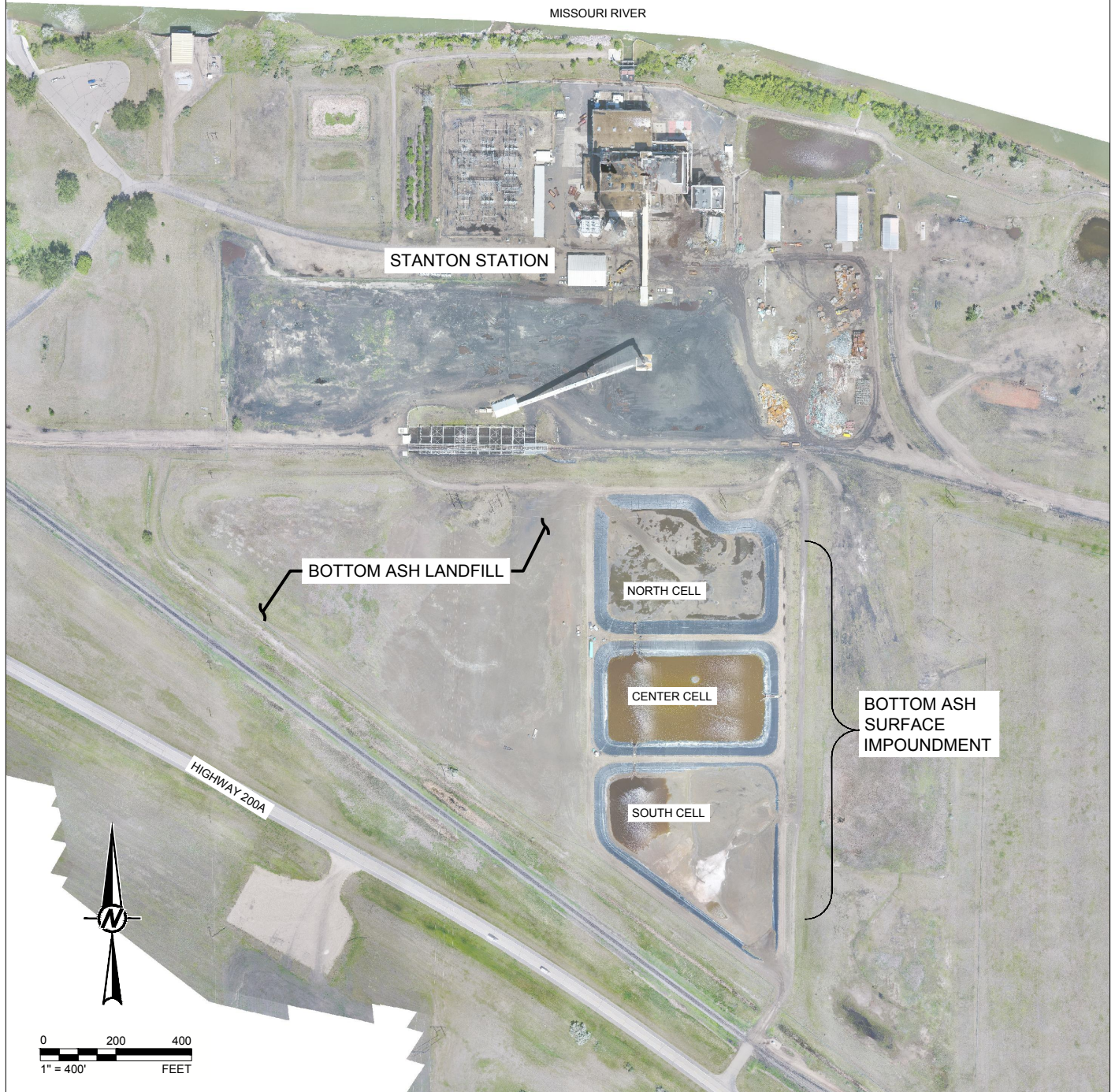
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[https://golderassociates.sharepoint.com/sites/23291g/deliverables/reports/ss\\_bai\\_annualinspection\\_fnl\\_08jan19/bai annual inspection\\_fnl\\_08jan19.docx](https://golderassociates.sharepoint.com/sites/23291g/deliverables/reports/ss_bai_annualinspection_fnl_08jan19/bai%20annual%20inspection_fnl_08jan19.docx)

## FIGURES



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#### REFERENCE(S)

1. AERIAL IMAGE FROM GREAT RIVER ENERGY PHOTOGRAPH JUNE 2018.

CLIENT  
GREAT RIVER ENERGY  
STANTON STATION  
STANTON, NORTH DAKOTA

CONSULTANT



YYYY-MM-DD 2018-12-19

DESIGNED KAC

PREPARED KAC

REVIEWED CCS

APPROVED CCS

PROJECT  
2018 ANNUAL CCR FACILITY INSPECTION REPORT

TITLE  
**STANTON STATION SITE OVERVIEW**

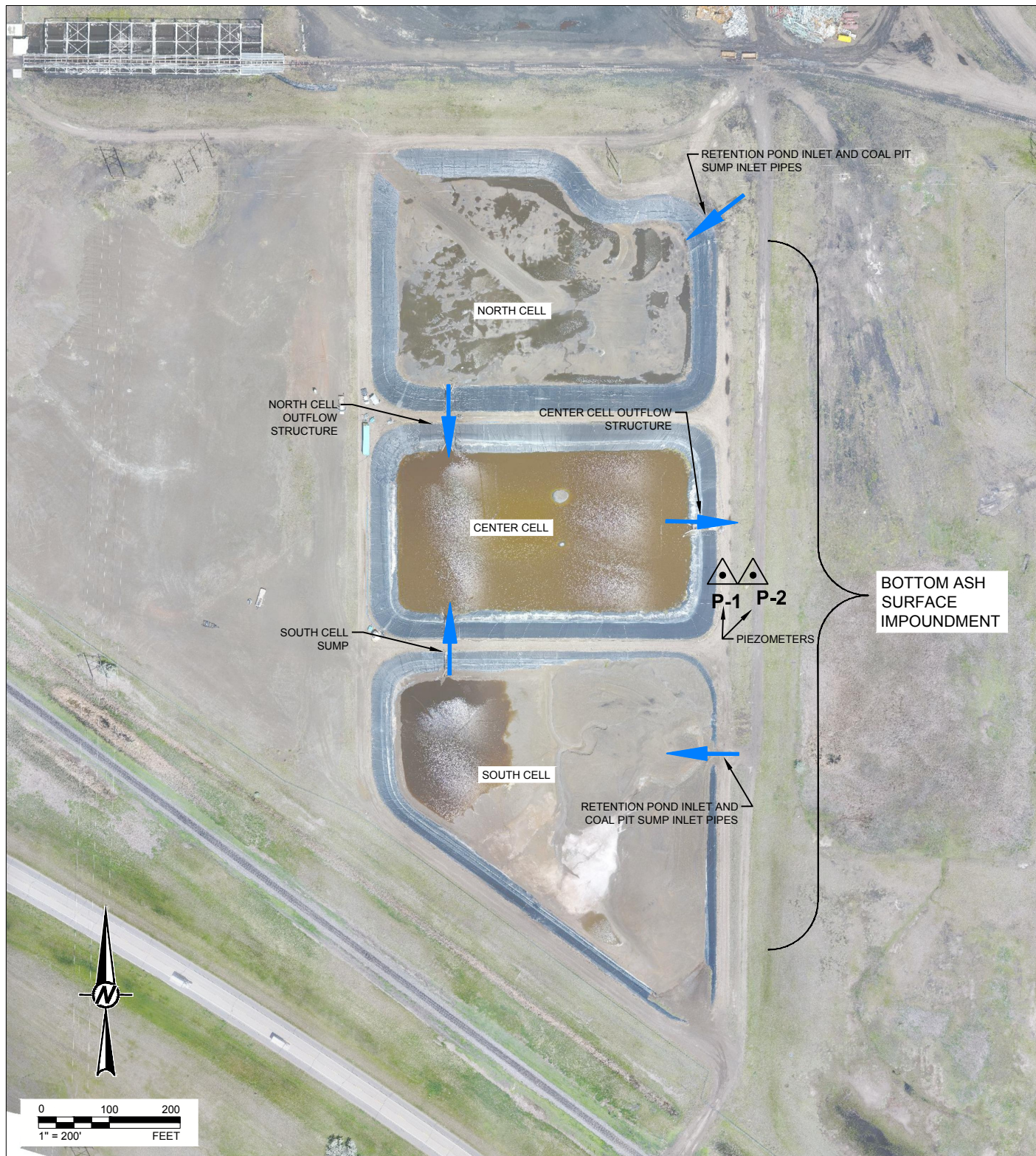
PROJECT NO.  
1894194

REV.  
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FIGURE  
1

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#### REFERENCE(S)

1. AERIAL IMAGE FROM GREAT RIVER ENERGY PHOTOGRAPH JUNE 2018.

CLIENT  
 GREAT RIVER ENERGY  
 STANTON STATION  
 STANTON, NORTH DAKOTA

CONSULTANT



YYYY-MM-DD 2018-12-19

DESIGNED KAC

PREPARED KAC

REVIEWED CCS

APPROVED CCS

PROJECT  
 2018 ANNUAL CCR FACILITY INSPECTION REPORT

TITLE  
**BOTTOM ASH IMPOUNDMENT SITE OVERVIEW**

PROJECT NO.  
 1894194

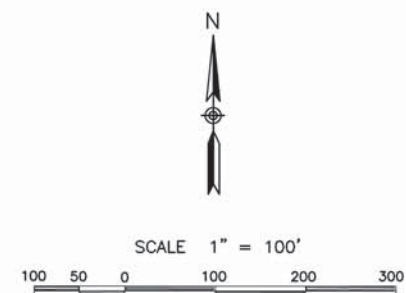
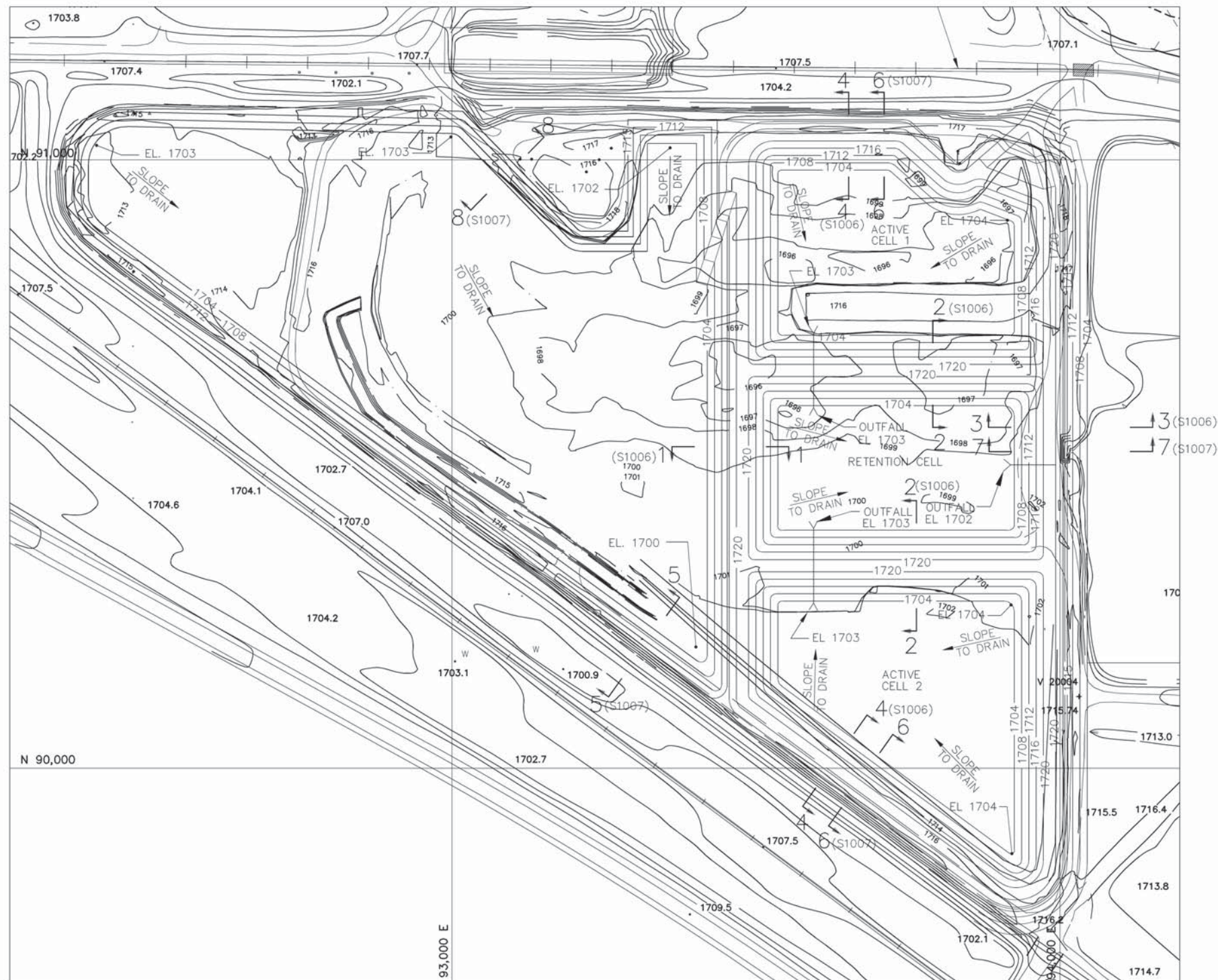
REV.  
 0

FIGURE  
 2

## **APPENDIX A**

### **Selected Construction Drawings and Permit Drawings**





LEGEND:

- EXISTING GROUND ELEVATION CONTOUR
- NEW GROUND ELEVATION CONTOUR

NOTES:

- SEE GENERAL NOTES, DRAWING S1002.
- ELEVATIONS SHOWN AT THE BOTTOM OF IMPOUNDMENT CELLS ARE OF THE TOP OF THE CLAY FILL.

REFERENCE DRAWINGS:

- S1002 FACILITIES SITE PLAN
- S1006 POND A SECTIONS & DETAILS - SH. 1
- S1007 POND A SECTIONS & DETAILS - SH. 2
- S1008 POND A SECTIONS & DETAILS - SH. 3
- S1009 OUTFALL STRUCTURES OUTLINE - PLAN & SECTIONS
- S1010 OUTFALL STRUCTURES - REINFORCEMENT SH. 1
- S1011 OUTFALL STRUCTURES - REINFORCEMENT SH. 2

3										2										1										0									
ISSUE										ISSUE										ISSUE										ISSUE									
PC										PC										PC										PC									
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CONC										CONC										CONC										CONC									
STL										STL										STL										STL									
LAP										LAP										LAP										LAP									
INST										INST										INST										INST									
MD										MD										MD										MD									
P.S.										P.S.										P.S.										P.S.									
S.A.										S.A.										S.A.										S.A.									
FAC										FAC										FAC										FAC									
ELEC										ELEC										ELEC										ELEC									
LTD										LTD										LTD										LTD									
MATL										MATL										MATL										MATL									

UPA PROJECT

4177

UNITED POWER ASSOCIATION  
STANTON STATION  
ASH POND MODIFICATIONS

POND A CONVERSION PLAN

DRAWING NO.

S1005

REV

1

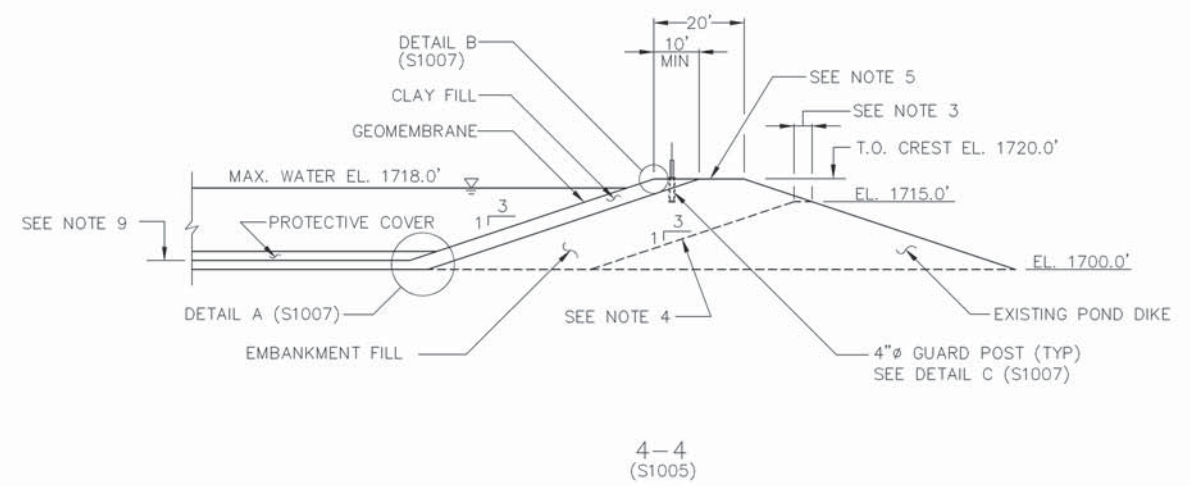
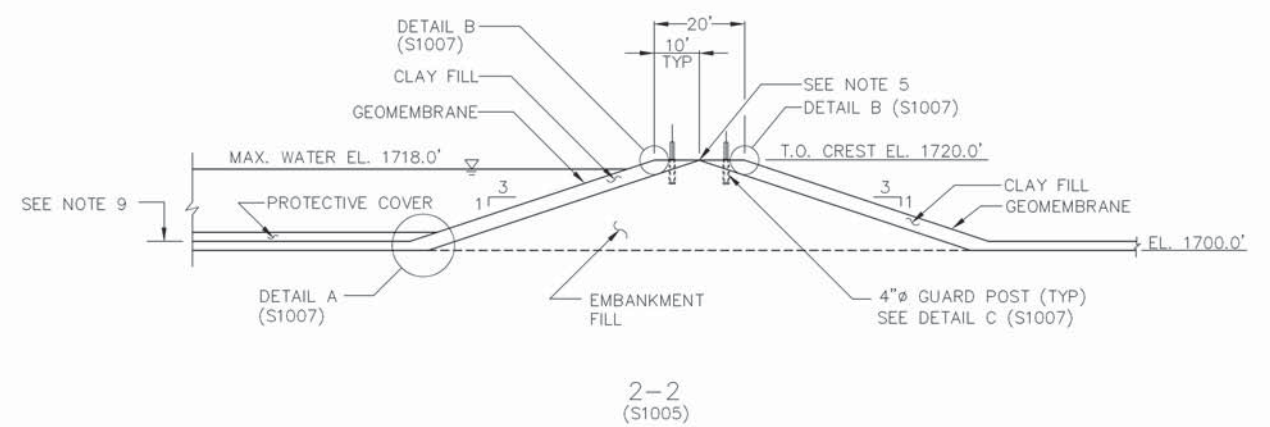
STONE & WEBSTER ENGINEERING CORPORATION  
DENVER, CO.

CAD FILE  
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HAUT-SIZE  
APP. CARD

ISSUE





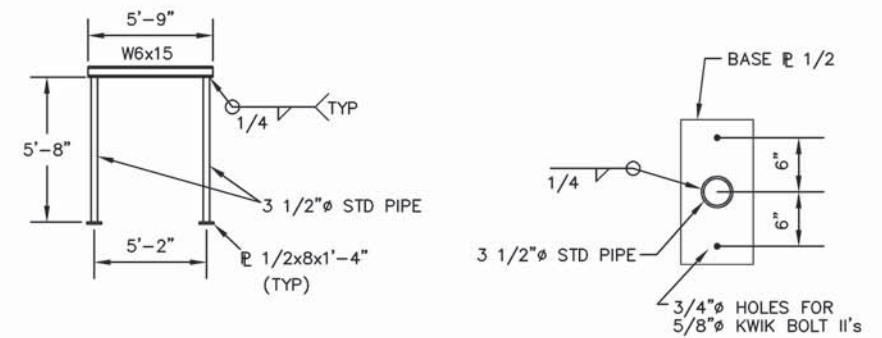
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	4177	S1006	
POND A SECTIONS & DETAILS - SH. 1			

**STONE & WEBSTER ENGINEERING CORPORATION**

DENVER, CO.

DATE: 11/1/84

BY: [Signature]



GRATING SECTION SHALL BE HINGED TO SWING UP AWAY FROM LADDER (SEE NOTE 11, DWG S1011)

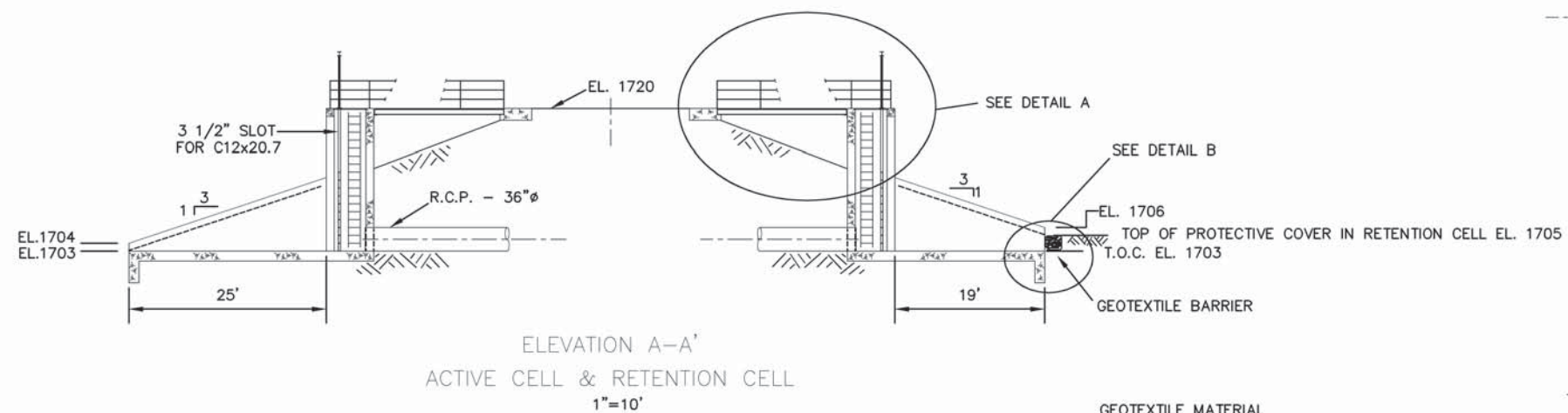
GRATING SECTION OVER SLOT SHALL BE HINGED TO SWING TOWARD THE WING WALLS (SEE NOTE 11, DWG S1011)

1 1/2" REMOVABLE GRATING COVER

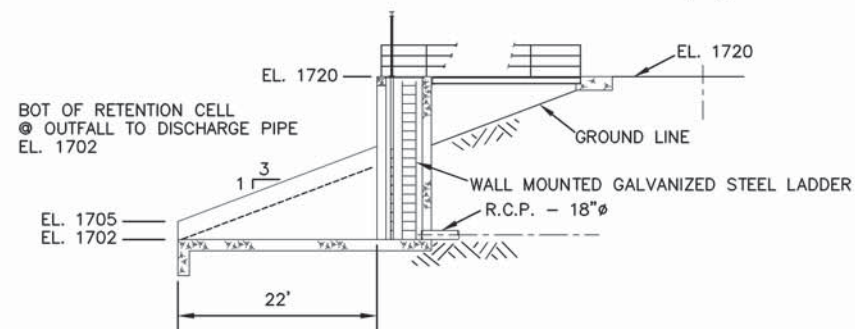
6'-0"

6'-0"

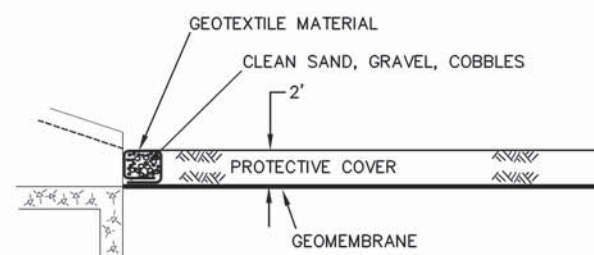
PARTIAL PLAN  
DETAIL A  
N.T.S.



ELEVATION A-A'  
ACTIVE CELL & RETENTION CELL  
1"=10'

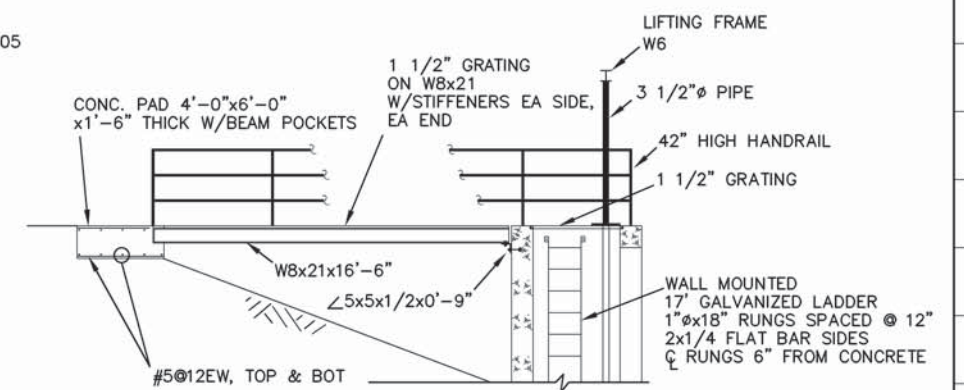


ELEVATION B-B'  
OUTFALL OF RETENTION CELL  
1"=10'



NOTE: THE GEOTEXTILE BARRIER SHALL EXTEND 2 FEET BEYOND THE RETENTION CELL OUTFALL WINGWALLS.

PARTIAL ELEV.  
DETAIL B  
N.T.S.



PARTIAL ELEV.  
DETAIL A  
N.T.S.

1. SCALE: AS NOTED
2. FOR ADDITIONAL NOTES, SEE DRAWING S1011

REFERENCE DRAWINGS:

S1011    OUTFALL STRUCTURES  
             REINFORCEMENT, SHEET 2

UPA PROJECT	UNITED POWER ASSOCIATION STANTON STATION ASH POND MODIFICATIONS	DRAWING NO.	REV 2
	4177	S1009	
OUTFALL STRUCTURES OUTLINE PLAN AND SECTIONS			

STONE & WEBSTER ENGINEERING CORPORATION

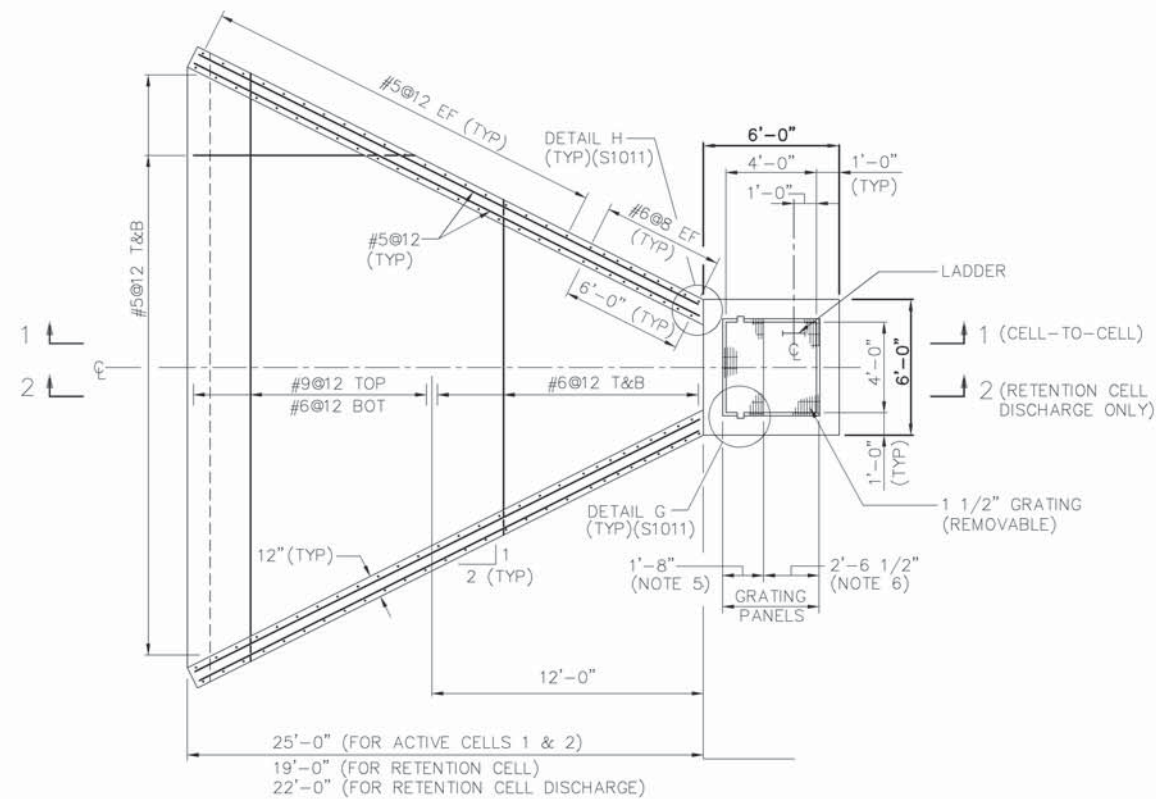
DENVER, CO.

DATE: 10/1/83

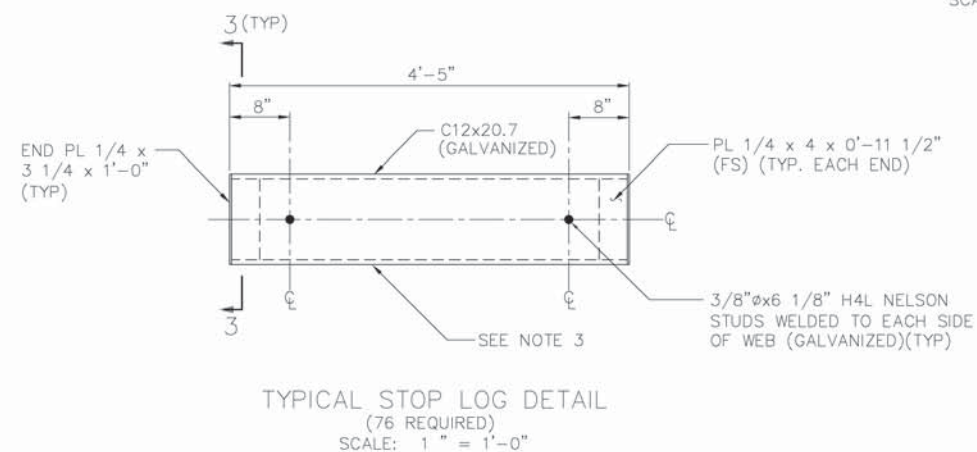
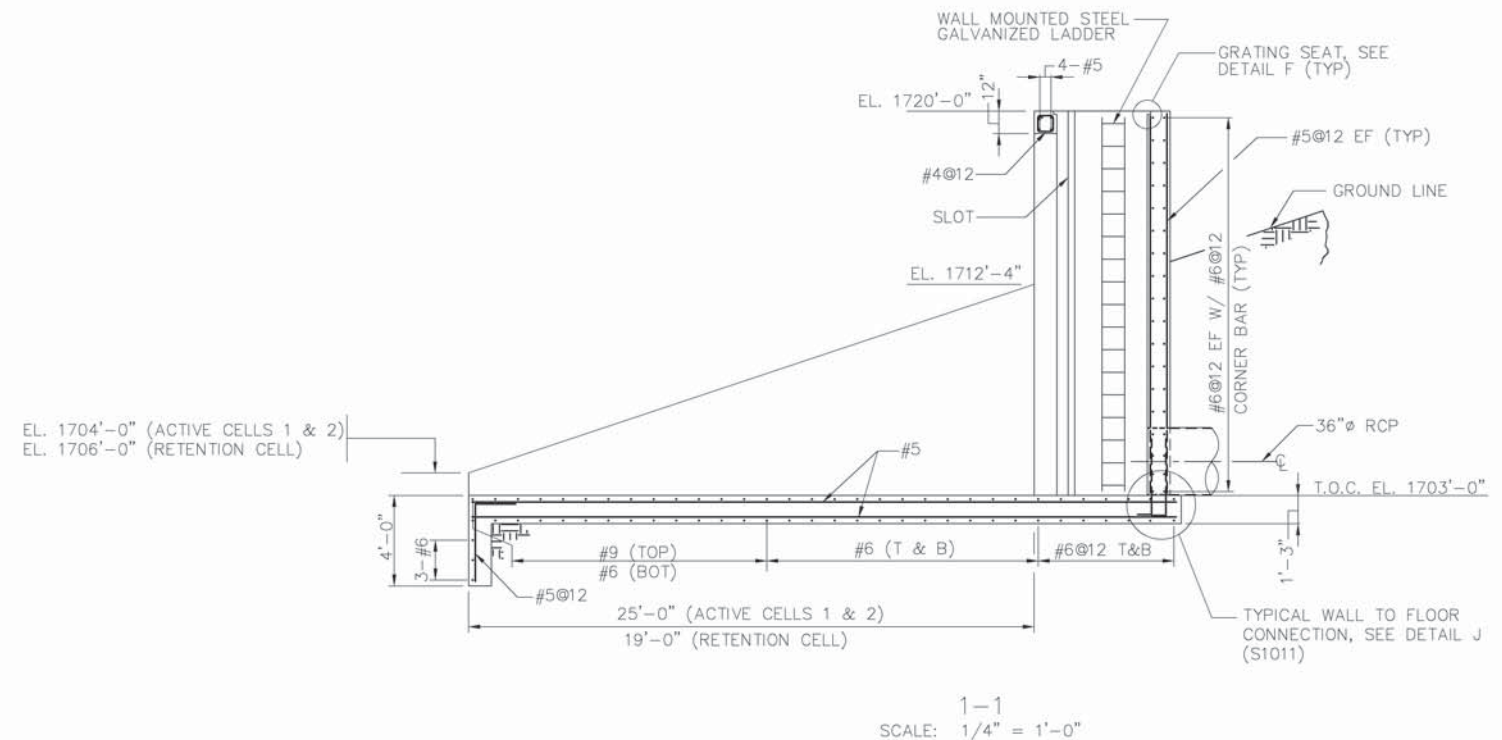
BY: [Signature]

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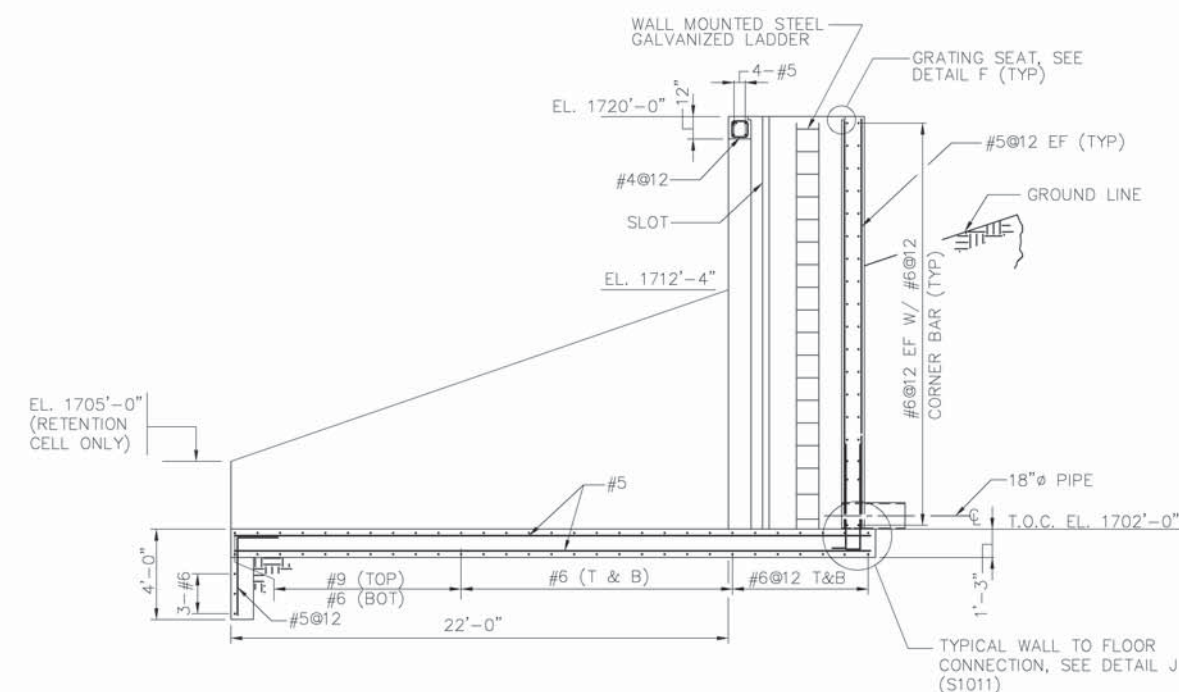


OUTFALL STRUCTURES - PLAN  
SCALE: 1/4" = 1'-0"

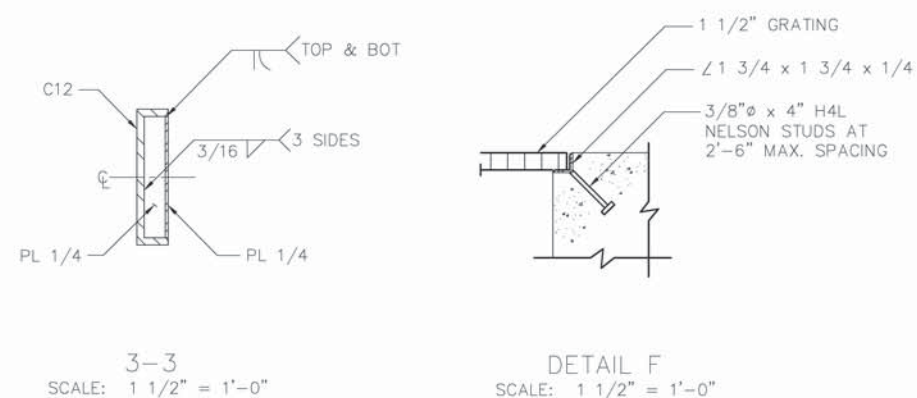


- NOTES:
- SCALE: AS NOTED
  - FOR ADDITIONAL NOTES, SEE DRAWING S1011.
  - PLACE 40 MIL x 2 1/2" STRIP PVC ON TOP AND BOTTOM OF C12 WITH WATER RESISTANT ADHESIVE.
  - THE FABRICATOR SHALL VERIFY AND CONNECT, AS REQUIRED, THE SWEEP OF THE CHANNEL STOP LOGS TO WITHIN 3/16", AND THE CAMBER TO WITHIN 1/16". THE VARIATION OF OUT OF SQUARE OF THE FLANGES SHALL BE VERIFIED TO BE LESS THAN THE 1/32" THAT IS PERMISSIBLE BY STANDARD MILL PRACTICE.
  - THE GRATING SECTION OVER THE SLOT SHALL BE HINGED TO SWING TOWARD THE WING WALLS.
  - THE GRATING SECTION SHALL BE HINGED TO SWING UP AWAY FROM THE LADDER.

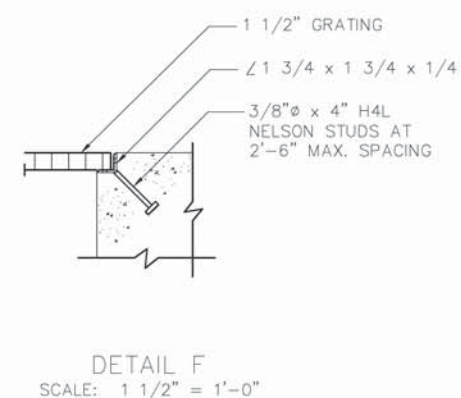
- REFERENCE DRAWINGS:
- S1009 OUTFALL STRUCTURES OUTLINE PLANS AND SECTIONS
  - S1011 OUTFALL STRUCTURES REINFORCEMENT, SHEET 2



2-2  
SCALE: 1/4" = 1'-0"



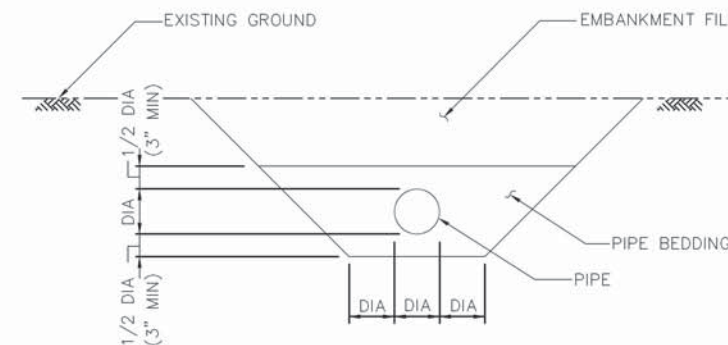
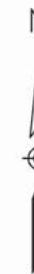
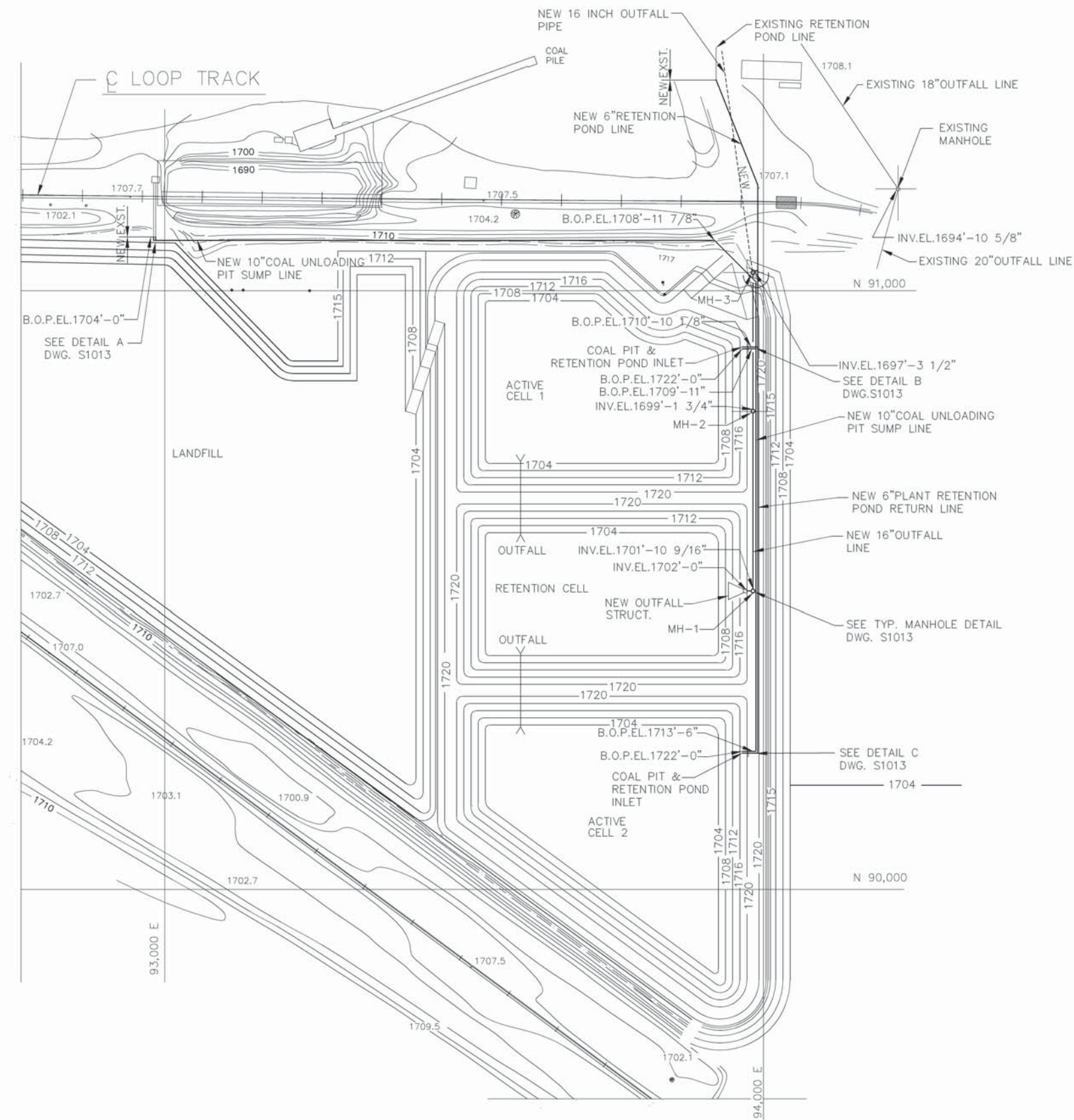
3-3  
SCALE: 1 1/2" = 1'-0"



DETAIL F  
SCALE: 1 1/2" = 1'-0"

3															2	FOR CONSTRUCTION REVISED PER ADDENDUM 1																												1	FOR CONSTRUCTION														WLZ														SGG —<													
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UPA PROJECT	UNITED POWER ASSOCIATION STANTON STATION ASH POND MODIFICATIONS	DRAWING NO.	REV
4177	OUTFALL STRUCTURES REINFORCEMENT - SH. 1	S1010	2
STONE & WEBSTER ENGINEERING CORPORATION DENVER, CO.			



PIPE AND CULVERT TRENCH DETAIL  
NTS

NOTE: PIPES AND/OR CULVERTS IN A SINGLE TRENCH SHALL HAVE A MINIMUM SPACING OF 12 INCHES (WALL-TO-WALL) BETWEEN ADJACENT PIPES.

NOTES:  
1. FOR GENERAL NOTES, SEE DRAWING S1002.

REFERENCE DRAWINGS:  
S1002 FACILITIES SITE PLAN  
S1013 PIPING DETAILS  
S1027 BOTTOM ASH SURFACE IMPOUNDMENT OUTFALL PIPING PLAN

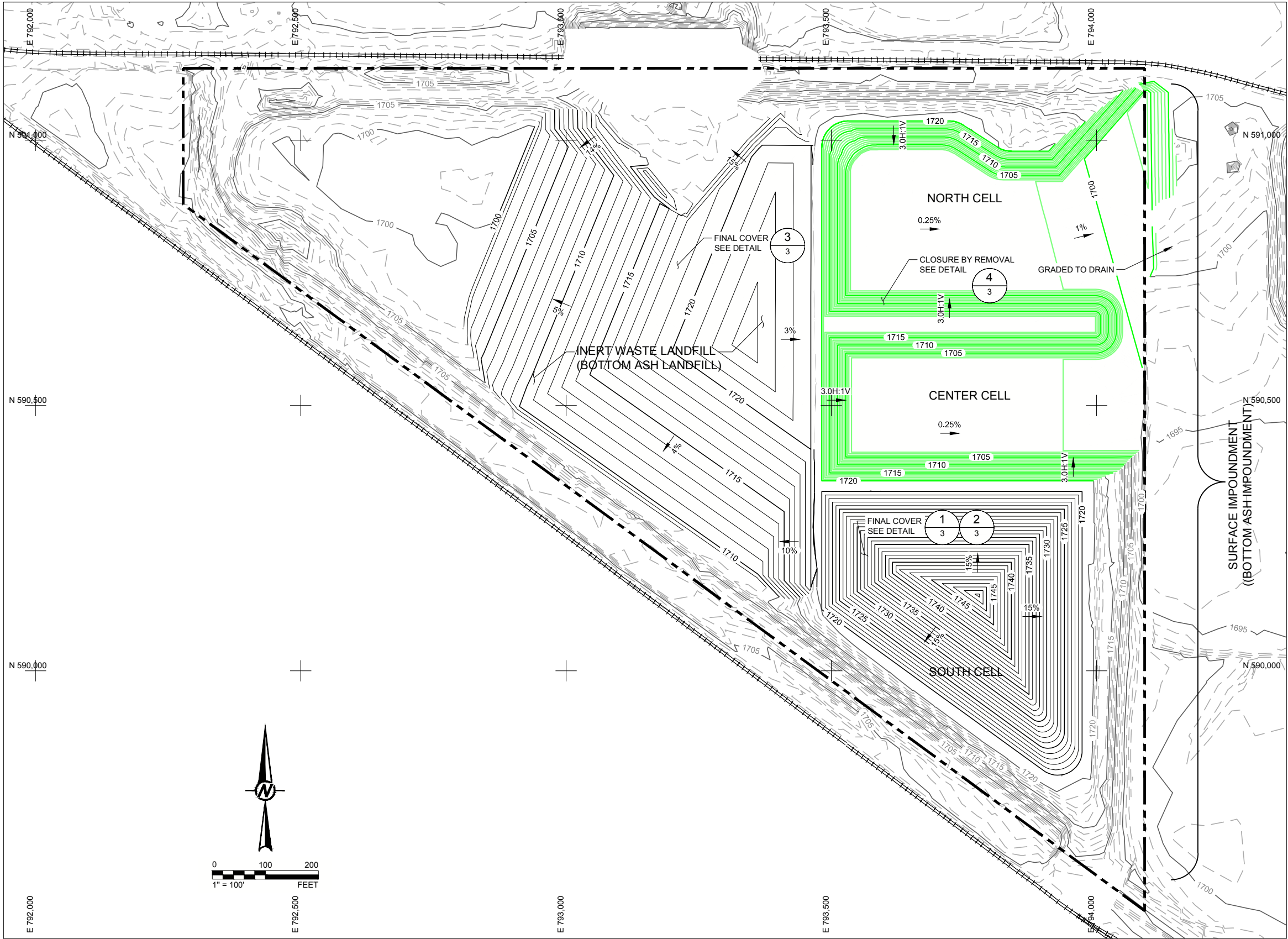
3											2	FOR CONSTRUCTION REVISED PER ADDENDUM 1																				1	FOR CONSTRUCTION										WVG — — — WZ — — — KWC/ WLZ — — — 6-9-94										0	ORIGINAL ISSUE FOR UPA REVIEW										KWC — — — WVG — — — KWC 4-19-																																	
ISSUE		DESCRIPTION										CHKD	CORRECT		APRIS		DATE		ISSUE		DESCRIPTION										CHKD	CORRECT		APRIS		DATE		ISSUE		DESCRIPTION										CHKD	CORRECT		APRIS		DATE		ISSUE		DESCRIPTION										CHKD	CORRECT		APRIS		DATE																							
PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL	PC	ARCH	CIVIL	CONC	STL	LAP	INST	MD	P.S.	S.A.	FAC	ELEC	LTD	MATL

UPA PROJECT 4177	UNITED POWER ASSOCIATION STANTON STATION ASH POND MODIFICATIONS	DRAWING NO. S1012	REV 2
	POND A PIPING PLAN		
<b>STONE &amp; WEBSTER ENGINEERING CORPORATION</b> DENVER, CO.			

ISSUE  
HAUF-SIZE  
APP. CARD  
UPA1012.DWG



Path: U:\Denver\ad\GREAT RIVER ENERGY\STANTON\09\_PROJECTS\1775717\Closure and Post-Closure Plan | File Name: 1775717A001.dwg | Last Edited By: cshuaitapz | Date: 2019-12-19 Time: 2:51:11 PM | Printed By: CShuaitapz | Date: 2019-12-21 Time: 11:02:39 AM



**LEGEND**

- 1705 EXISTING GROUND TOPOGRAPHY (REFERENCE 2)
- 1705 PROPOSED FINAL CLOSED BY REMOVAL GRADES
- 1735 PROPOSED FINAL COVER GRADES (NOTE 2)
- EXISTING FACILITY BOUNDARY (NOTE 1)
- RAIL LINES

**NOTES**

- THE FACILITY BOUNDARY IS APPROXIMATE.
- FINAL TOP OF COVER GRADES WILL NOT EXCEED 15 PERCENT OR BE LESS THAN 3 PERCENT PER NDAC RULES.

**REFERENCES**

- SITE LOCATION: SECTIONS 16 AND 21, T144N, R84W, MERCER COUNTY, NORTH DAKOTA.
- EXISTING GROUND TOPOGRAPHY WAS ACQUIRED BY INTERSTATE ENGINEERING IN 2014, 2017, AND 2018.
- COORDINATES ARE BASED ON THE PLANT GRID SYSTEM.
- THE CONTOUR INTERVAL IS ONE FOOT.
- THE INERT WASTE LANDFILL AND SURFACE IMPOUNDMENT FACILITIES ARE CONTAINED WITHIN PROPERTY OWNED BY GREAT RIVER ENERGY.

C	2018-12-20	REVISED ISSUED FOR CLOSURE PLAN	CCS	CCS	TJS	TJS
B	2018-11-20	ISSUED FOR CLOSURE PLAN	CCS	CCS	TJS	TJS
A	2018-09-12	ISSUED FOR CLIENT REVIEW	CCS	CCS	TJS	TJS
REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED

CLIENT  
GREAT RIVER ENERGY  
STANTON STATION  
STANTON, NORTH DAKOTA

CONSULTANT



GOLDER ASSOCIATES INC.  
44 UNION BLVD., SUITE 300  
LAKEWOOD, COLORADO  
USA  
(303) 980-0540  
www.golder.com

PROJECT  
SP-043 CLOSURE PLAN

TITLE  
**TOP OF FINAL COVER AND CLOSURE PLANS**  
*BOTTOM ASH LANDFILL AND BOTTOM ASH IMPOUNDMENT*

PROJECT NO. 1775717 REV. C 1 of 3 DRAWING 1

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM A3 AND D

## **APPENDIX B**

### **Visual Observations Checklist**

## IMPOUNDMENT INSPECTION CHECKLIST

**Facility Name:** Bottom Ash Impoundment

**Owner and Address:** Great River Energy – Stanton Station

**Purpose of Facility:** CCR Dewatering and process water storage clarification

**Legal:** Section 21

**Township:** 144N

**Range:** 84W

**County:** Mercer

**Inspected By:** Todd Stong, Craig Schuettpelz, Paul Schlicht **Inspection Date:** September 18, 2018

**Weather:** Sunny/Fog, 40°F - 60°F, Low wind, No Precipitation

ITEM	Y	N	N/A	REMARKS
1. Water Levels				
a. High water mark			X	El: N/A
b. Current water level	X			El: North: N/A, Center = N/A, South = 1710'
2. Inflow Structure				Not in use at time of inspection
a. Settlement		X		
b. Cracking		X		
c. Corrosion		X		Bottom Ash piping removed
d. Obstacles in inlet		X		
e. Riprap/erosion control	X			Geomembrane rub sheet at inlet pipe locations
3. Outflow Structure				Not in use at time of inspection
a. Settlement		X		
b. Cracking		X		
c. Corrosion		X		
d. Obstacles in outlet		X		
e. Riprap/erosion control			X	
4. Upstream slope				
a. Erosion – liner exposed?	X			Liner is exposed on side slopes, air trapped below liner at several locations on the floor of the center cell
b. Rodent burrows		X		
c. Vegetation	X			Minor vegetation along toe where sediment has accumulated
d. Cracks/settlement	X			Soil underlying geomembrane has moved at some locations
e. Riprap/other erosion protection			X	
5. Crest				
a. Soil condition	X			Gravel road, no significant settlement/cracking
b. Comparable to design width	X			
c. Vegetation	X			Weed growth within gravel crest roads
d. Rodent burrows		X		
e. Exposed to heavy traffic		X		Facility is currently inactive
f. Damage from vehicles/machinery		X		
6. Downstream slope				
a. Erosion	X			Minor erosion
b. Vegetation	X			Grass, few bare spots, minor woody vegetation
c. Rodent burrows	X			Small to medium sized burrows
d. Cracks/settlement/scarps		X		
e. Drain conditions			X	
f. Seepage		X		
7. Toe				
a. Vegetation	X			Grass
b. Rodent burrows	X			Small to medium sized burrows
c. Settlement		X		
d. Drainage conditions	X			Surface water drainages/ponding areas
e. Seepage		X		

**General Remarks:** Some movement of upstream slopes due to pumping of soil under geomembrane liners, but no signs of instability. Ongoing maintenance to control/repair rodent burrows and minor erosion. Water found between clay and geomembrane near toe of cells. Air trapped under the geomembrane on the floor of the center cell.

**Name of Engineer:** Craig Schuettpelz

**Date:** 9/18/2018

**Engineering Firm:** Golder Associates Inc.

**Signature:** 



PROFESSIONAL ENGINEER SEAL

## **APPENDIX C**

### **Photographs**





**LEGEND**

# PHOTOGRAPH NUMBER AND DIRECTION

**REFERENCES**

1. AERIAL IMAGE FROM GREAT RIVER ENERGY PHOTOGRAPH TAKEN JUNE 2018.

CLIENT  
GREAT RIVER ENERGY  
STANTON STATION  
STANTON, NORTH DAKOTA

CONSULTANT



YYYY-MM-DD	2018-12-19
DESIGNED	KAC
PREPARED	KAC
REVIEWED	CCS
APPROVED	CCS

PROJECT  
2018 ANNUAL CCR FACILITY INSPECTION REPORT

TITLE  
**BOTTOM ASH IMPOUNDMENT  
PHOTOGRAPH LOCATIONS**

PROJECT NO.  
1894194

REV.  
A

FIGURE  
1



**Bottom Ash Impoundment**



Photograph 1 (West downstream slope)  
Grass vegetation (IMGP6232.JPG)



Photograph 2 (SE corner)  
Fence around perimeter of Bottom Ash Impoundment, partially damaged (separated from fence post)  
(IMGP6235.JPG)

**Bottom Ash Impoundment**



Photograph 3 (East side)  
Woody vegetation on slope (IMGP6238.JPG)



Photograph 4 (East downstream slope)  
Grass vegetation on east berm downstream slope (IMGP6239.JPG)



**Bottom Ash Impoundment**



Photograph 5 (East side)  
Mid-slope road and upper east downstream slope (IMGP6240.JPG)



Photograph 6 (South Cell berm crest)  
Upstream slope and interior of South Cell (IMGP6253.JPG)



Bottom Ash Impoundment



Photograph 7 (North/Center Cell berm crest)  
Center/North Cell crest with gravel road surface (1ccs.JPG)



Photograph 8 (Center Cell upstream slope)  
Upstream slope and toe of Center Cell (3ccs.JPG)



Bottom Ash Impoundment



Photograph 9 (East berm crest)  
Upstream slope and interior of Center Cell (8ccs.JPG)



Photograph 10 (East downstream slope)  
Erosion and rutting on downstream slope near the toe (9ccs.JPG)



Bottom Ash Impoundment



Photograph 11 (East toe)  
Grass vegetation at the toe of the slope (10ccs.JPG)



Photograph 12 (East toe)  
Animal burrows at the toe of the slope (12ccs.JPG)



Bottom Ash Impoundment



Photograph 13 (Center/South Cell berm crest)  
Center/South Cell crest with gravel road surface (15ccs.JPG)



Photograph 14 (Center Cell east upstream slope)  
Failed geomembrane extrusion bead repair (16ccs.JPG)



Bottom Ash Impoundment



Photograph 15 (West berm crest)  
West crest with gravel road surface (21ccs.JPG)



Photograph 16 (South downstream slope)  
Grass vegetation on the south slope (25ccs.JPG)



Bottom Ash Impoundment



Photograph 17 (South downstream slope)  
Animal burrow on south downstream slope (29ccs.JPG)



Photograph 18 (North Cell east upstream slope)  
Minor geomembrane damage on upstream slope and retention pond/coal pit sump inlet piping (SS PDS (6).JPG)



Bottom Ash Impoundment



Photograph 19 (East berm crest)  
East berm crest with gravel road surface (SS PDS (8)).JPG)



Photograph 20 (North downstream slope)  
Grass vegetation and animal burrows on the north downstream slope (SS PDS (29)).JPG)



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