



REPORT

Annual Inspection

Coal Creek Station – Upstream Raise 92 CCR Surface Impoundment

Submitted to:

Great River Energy

2875 Third Street SW
Underwood, North Dakota 58576

Submitted by:

Golder Associates Inc.

44 Union Boulevard, Suite 300 Lakewood, Colorado, USA 80228

+1 303 980-0540

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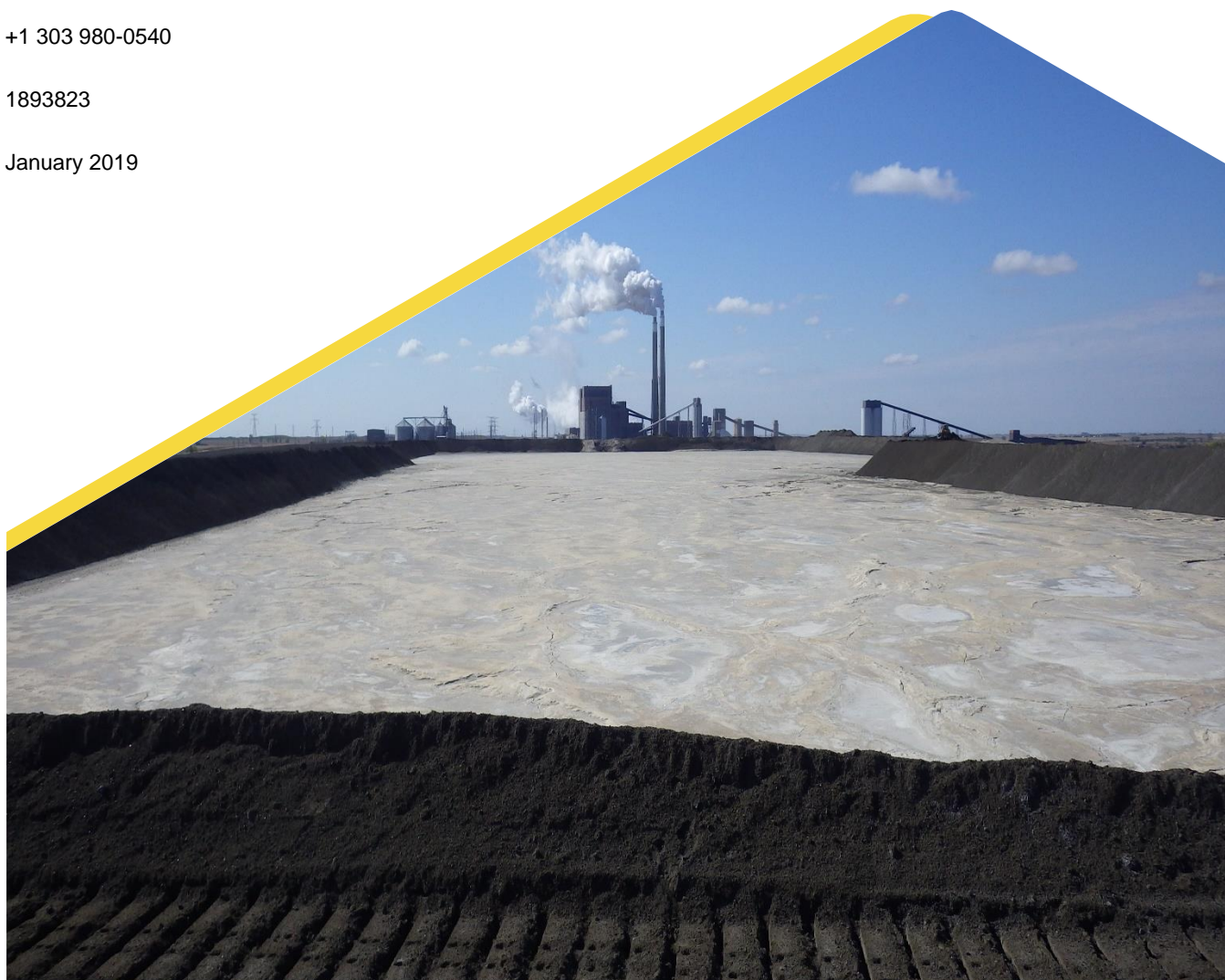


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1.0 INTRODUCTION

As part of 40 CFR Part 257 of the Subtitle D solid waste provisions under the Resource Conservation and Recovery Act (RCRA), utilities are required to complete annual inspections for surface impoundments and landfills containing Coal Combustion Residuals (CCR). This report has been prepared by Golder Associates Inc. (Golder) for Great River Energy (GRE) to satisfy the annual inspection requirements for CCR surface impoundments under 40 CFR Part 257.83.

Coal Creek Station (CCS) is located in McLean County, approximately 10 miles northwest of Washburn, North Dakota. There are four facilities located at CCS that fall under the CCR rule requirements (Figure 1):

- Drains Pond System CCR Surface Impoundment (Drains Pond System)
- Upstream Raise 91 CCR Surface Impoundment (Upstream Raise 91)
- Upstream Raise 92 CCR Surface Impoundment (Upstream Raise 92)
- Southeast Section 16 CCR Landfill (Southeast 16)

Upstream Raise 91 and Upstream Raise 92 both operate as impoundments and will be closed with CCR in-place. The Drains Pond System is currently being used to dewater bottom ash and as a process water impoundment to return conveyance water back to the plant. The Southeast 16 landfill operates as a landfill and is used as a storage/disposal facility for CCRs that do not contain free liquid. This report presents a review of available facility information and findings of the inspection of Upstream Raise 92 at CCS performed September 19, 2018.

2.0 REVIEW OF EXISTING INFORMATION

2.1 Geological Conditions

Upstream Raise 92 is generally constructed over a glacial till layer consisting of sandy and silty-clay soils. Glacial till varies in thickness from 20 feet to several hundred feet in the area of Coal Creek Station. Silty-sand and sand lenses are present throughout the glacial till formation, which is underlain by poorly consolidated siltstone/sandstone bedrock (Barr Engineering 1982; CPA and UPA 1989).

2.2 Facility Location and Operation

Upstream Raise 92 (Figure 2) is located in Section 16, Township 145N, Range 82W and covers approximately 110 acres. The facility is used as a combined dewatering and storage facility for CCRs including fly ash, bottom ash, and flue gas desulfurization (FGD) material. FGD material and hydraulic conveyance water enter Upstream Raise 92 through a high-density polyethylene (HDPE) pipe from the plant to the northwest corner of the facility. The pipe runs above ground from the northwest corner of the facility to the final discharge location. The on-grade HDPE pipe is periodically moved to different areas of Upstream Raise 92 to achieve an even distribution of FGD material in the facility. Bottom ash and fly ash are hauled to the facility using heavy construction equipment, such as Caterpillar 777 haul trucks. The facility is dewatered using gravity-driven drainage pipes that extend from the west side of Upstream Raise 92 to the east side of Upstream Raise 91. Upstream Raise 92 is approximately 300 feet south of Lower Samuelson Slough and 100 feet north of the rail lines. Upstream Raise 91 is adjacent to the west end of the facility and the plant dry CCR landfill (Southeast 16) is adjacent to the east side of the facility. Upstream Raise 92 was constructed within the boundaries of the historic Ash Pond 92 (west side of Upstream Raise 92) and the Southwest Section 16 Landfill (east side of Upstream Raise 92).

2.3 Site History and Liner Systems

Ash Pond 92 was originally part of the South Ash Pond. The South Ash Pond was constructed with a clay core dike around the perimeter and a soil liner. A new clay liner was installed over the South Ash Pond in 1982 and the facility remained in operation until 1987 when in-place CCRs were excavated from the South Ash Pond and transported to the Section 5 dry CCR landfill (Eugene A. Hickok & Associates 1986; Foth & Van Dyke 1988). The South Ash Pond was then divided into Ash Pond 91 and Ash Pond 92. Ash Pond 92 was deepened and a new composite liner consisting of a 2-foot thick clay and 40-mil HDPE liner was completed in 1989. The liner is overlain with 1 foot of sand, 1 foot of Pit Run gravel, and a drainage system with collection pipes that slope to the north side of the facility. Ash Pond 92 was modified in 2002 to allow for “vertical” placement of CCRs. Since September of 2002, the facility has been constructed with an interior area of FGD material, a drainage layer of bottom ash, and an outer shell of fly ash (Figure 3).

The Southwest Section 16 Landfill was originally part of the East Ash Pond and was constructed with a natural clay liner. In 1989, the facility was reclassified as a solid waste disposal area and CCRs from the other parts of the East Ash Pond were excavated and placed in the Southwest Section 16 Landfill. The Southwest Section 16 Landfill was re-graded and a new composite liner consisting of a 1-foot thick clay and 60-mil LLDPE liner was installed over the previously placed CCR in three phases between 2005 and 2008. The liner is overlain with a “liner head reduction system” consisting of 18 inches of granular material and drainage pipes overlain by a 1-foot clay liner. The Southwest Section 16 Landfill was re-graded and relined to allow for “vertical” placement of CCRs and has been connected with the “vertical” placement of CCRs occurring in Ash Pond 92. An additional 7 acres of composite liner was installed in the southeast corner of Upstream Raise 91 and in the area between Upstream Raise 91 and Upstream Raise 92 in 2016. The liner completes a continuous composite-lined area between Upstream Raise 91 and Upstream Raise 92. The composite liner system installed in 2016 consists of (from bottom to top): Geosynthetic Clay Liner (GCL) and 60-mil HDPE liner.

Appendix A contains additional information regarding the design of Upstream Raise 92.

2.4 Site Geometry

The design crest of the original soil perimeter berms surrounding Upstream Raise 92 are at approximate elevations between 1900 feet above mean sea level (amsl) and 1920 feet amsl. This berm surrounding the facility has a gravel surfaced roadway supporting both light passenger vehicles and heavy construction equipment, such as Caterpillar 777 haul trucks. Based on existing topography, original berm downstream slopes generally have 3:1 or 2.5:1 slopes down to perimeter drainage ditches with elevations between approximately 1880 feet amsl and 1900 feet amsl. Original soil perimeter berm upstream slopes have an approximate 3:1 slope from this original soil perimeter berm to the base of the facility between 1892 feet amsl and 1910 feet amsl.

An expansion berm on the east half of the facility built with soil and CCRs extends from the original soil perimeter berms to an elevation of 1950 feet amsl at 3.5:1 to 4:1 slopes. The entire facility is designed with 4:1 final CCR slopes from the perimeter berms to elevation 1974 feet amsl, 15% final CCR grades between elevations 1974 feet amsl and 2004 feet amsl, and a 5% crown to achieve a final CCR elevation of approximately 2022 feet amsl (Figure 3).

2.5 Changes in Geometry

No significant changes to geometry were noted other than the continued placement of CCRs to the design grades. Between January and December of 2018, the bottom ash/fly ash has increased in elevation from

approximately 1992 feet amsl to 1995 feet amsl and the FGD material has increased in elevation from approximately 1980 feet amsl to 1982 feet amsl.

2.6 Storage Capacity and Volumes

Based on a comparison between the approximate grades as of the fall of 2018 and the final permitted grades of Upstream Raise 92, the facility has a remaining CCR capacity of approximately 950,000 cubic yards (CY). The approximate total CCR capacity of Upstream Raise 92 is 12,920,000 CY. Therefore, the amount of CCR contained in the facility at the time of the inspection is estimated to be approximately 11,970,000 CY.

2.7 Impounded Water

The depth of impounded water in Upstream Raise 92 varies with time as more CCRs are deposited and as operational variables change (such as gravity drainage pipe elevations). Since FGD material was deposited in Upstream Raise 91 for a majority of 2018, Upstream Raise 92 did not contain a substantial amount of ponded water at the time of the inspection.

2.8 Permits

Upstream Raise 92 is currently permitted with the North Dakota Department of Health (NDDH) under Permit Number 0033. Previous permit modification documents describe additional historical information about the design of the facility (CPA 1997, CPA and UPA 1989, GRE 2003, GRE 2012).

2.9 Summary of 2018 Weekly Inspections

Routine weekly inspections of Upstream Raise 92 facility were performed throughout 2018 as a part of the final CCR Rule. Based on a review of the available inspection forms, the following items were noted:

- Generally good site maintenance.
- No signs of significant seepage, settlement, or cracking of the berm downstream slopes.
- Cracks in bottom ash and fly ash crests and side slopes were noted and onsite staff discussed the cracks with the design engineers. These cracks were attributed to consolidation of interior FGD materials and are expected to continue during placement of CCRs.
- Fugitive dust actively controlled using a water truck (as required).
- Damaged piezometers were noted in January 2018 due to ice and/or snow buildup and were repaired in the spring/summer of 2018.

2.10 Summary of Previous Inspections

The most recent annual professional engineer inspection of Upstream Raise 92 was performed by Golder in the fall of 2017 (Golder 2018) and a summary of the observations of that inspection are as follows:

- Generally good vegetation and site maintenance.
- No signs of significant or unexpected seepage, settlement, or cracking of the berm downstream slopes, although the surface water drainage at the toe along the south side of Upstream Raise 92 had some standing water and marshy vegetation due to recent grading activities.

- Cracks in bottom ash and fly ash CCR crests were noted and were attributed to consolidation of interior FGD materials. These cracks were expected to continue during placement of CCRs.
- Minor erosion of the fly ash “shell” (within the lined footprint of the facility).
- Several small animal burrows, but none that were anticipated to cause areas of structural weakness.
- Portions of the final cover seeded in the past three years have fair to good native grass vegetative growth.

3.0 2018 ANNUAL INSPECTION

On September 19, 2018, Todd Stong, Paul Schlicht, and Craig Schuettepelz of Golder performed a visual inspection of Upstream Raise 92 per USEPA Regulation 40 CFR Part 257.83(b) requirements. The inspection consisted of visual observations while walking around the facility traversing up and down the perimeter berm and CCR placement areas. An annual inspection checklist used during the inspection is presented in Appendix B. Photographs were taken and are presented in Appendix C. The following presents a summary of the observations made during the 2018 annual inspection.

3.1 Hydraulic Structures

Upstream Raise 92 has an inflow pipe for depositing FGD material; however, FGD material was being deposited in Upstream Raise 91 at the time of the inspection. The on-grade HDPE pipe is 8 inches in diameter and is periodically moved to different areas of Upstream Raise 92 (or Upstream Raise 91) to achieve an even distribution of FGD material in the facilities.

The outflows from Upstream Raise 92 consist of a series of gravity drainage pipes and culverts that transfer CCR conveyance water from the facility to the adjacent Upstream Raise 91. Over time, the gravity drainage pipes can become clogged with material and new pipes are installed to convey water between the facilities. At the time of the inspection, gravity drainage pipes were not operating, but appeared to be in good condition. Similarly, the culverts connecting the drainage ditch on the west side of Upstream Raise 92 to Upstream Raise 91 were in good condition.

The inflow and outflow systems appear to be in good condition with no sign of settlement, cracking, or corrosion.

3.2 Perimeter Berm

3.2.1 Berm Upstream Slope

The berm upstream slopes are mostly covered by CCR deposition and/or final cover. A small amount of the berm upstream slope from elevation 1917 to 1920 was visible on the north side of the facility. The observed slope appeared to match the design slopes of 3:1 and are being protected from erosion with a cemented fly ash layer. The berm upstream slopes appeared to be competent with no signs of significant distress.

3.2.2 Berm Crest

The berm crest along the north and south sides of Upstream Raise 92 is surfaced with gravel and used for both light vehicle and heavy construction equipment traffic. The berm crest on the west side of Upstream Raise 92 was not visible during the inspection. The crest of the original perimeter soil berm on the north and south sides of the facility (elevation 1900 feet amsl to 1920 feet amsl) is a gravel surfaced road that appeared to be in good condition. The roads were well-compacted and the north side experiences frequent heavy traffic. The visual inspections did not reveal signs of cracking, erosion, or settlement.

3.2.3 Berm Downstream Slope

The berm downstream slopes of Upstream Raise 92 below the original and expansion berms are heavily vegetated with native grasses. Occasional small animal burrows up to approximately two inches in diameter were observed on the north and south berm downstream slopes. There is no noticeable significant erosion, cracks, or scarps on these grassy slopes and they appear to be in good condition.

3.3 Toe

The toe of the slopes on the north and south sides of Upstream Raise 92 are mostly covered with tall grass. A few small animal burrows were noticed during the inspection, but there were no noticeable signs of seepage, cracks, or settlement.

The surface water drainage at the toe of the slope on the south side of Upstream Raise 92 had some marshy vegetation and standing water. The surface water drainage ditch in this area was recently modified to promote flow toward site surface water drainages and therefore portions of this ditch were not yet well-vegetated. The presence of some standing water in this surface water drainage ditch was still noted due to the low average slope of this ditch and the fact that it is near groundwater levels. Side slopes of this drainage ditch are also relatively steep near the toe, but did not show signs of movement. The toe may need to be regraded and reseeded in the future to limit erosion. In addition, the downchute channels should be extended and hydraulic jump basins constructed on the south side of the facility to limit erosion when final cover is constructed over the entirety of Upstream Raise 92.

3.4 CCR Placement

3.4.1 CCR Upstream Slope

The CCR upstream slope is defined as the slope that toes out into the raise pool. The CCR upstream slope of Upstream Raise 92 is constantly changing as bottom ash and FGD material are deposited. Therefore, the CCR upstream slopes are temporary and dependent on the angle of repose of the bottom ash material. The vertical distance from the top of the bottom ash CCR upstream slope to the water/FGD material mixture is approximately 9 to 12 feet in most locations. The CCR upstream slopes appear to be in good condition with no signs of structural weakness.

3.4.2 CCR Crest

The CCR crest along the top of the facility is constructed mainly of bottom ash. The CCR crest is bordered on the outsides of the facility by a fly ash “shell” primarily for erosion protection and as a trafficking surface. Bottom ash on the CCR crest of the facility is in good condition and is continually worked and compacted with heavy equipment. During the inspection, cracks were noted on the surface of the CCR downstream slopes and the CCR crest (between elevations 1974 feet amsl and 1995 feet amsl) within the lined footprint of the facility. In each case, the cracks were between approximately 1 and 18 inches wide, up to several feet deep, and up to several hundred feet long. The cracks are expected and can be attributed to consolidation of FGD material on the interior of the facility and the relatively rigid bottom ash and fly ash exterior of the facility. These cracks are continually observed and evaluated by onsite operations personnel for changes to the shape, offset, or length of the features, and the installed instrumentation (inclinometers and piezometers) provides additional information regarding the performance of the facility with respect to the design.

3.4.3 CCR Downstream Slope (no Cover)

The area above the original and expansion berm downstream slopes surrounding the west and east sides, and a portion of the north and south sides of Upstream Raise 92 had an exposed fly ash “shell” at the time of the inspections (CCR downstream slope). The fly ash CCR downstream slope is in good condition and there was no noticeable seepage, sloughing, cracking, or abnormally thriving vegetation, or settlement during the inspections; however, there was some minor erosion of the fly ash shell. The eroded fly ash is collected within the lined footprint in a perimeter ditch and must be periodically cleaned out as required.

3.4.4 CCR Downstream Slope (with Cover)

Portions of CCR downstream slopes on the north side of Upstream Raise 92 have temporary cover. These areas were well vegetated with grass and weeds but had experienced some minor erosion. The CCR downstream slopes with temporary cover appeared to be in generally good condition to aid in controlling erosion of the outer fly ash “shell” and limit wind-blown fugitive dust.

The south CCR downstream slope of Upstream Raise 92 has final cover on the side slopes to an elevation of 1974 feet amsl with terraces approximately every 20 vertical feet and downchute drainage channels along the side slopes. Final cover seeded in the past three years has fair to good native grass vegetative growth. These areas should continue to be monitored and should be re-seeded to promote growth of native grass species, as required. Other areas of the CCR downstream slope with final cover covered more than three years ago show good native grass vegetative growth.

3.5 Instrumentation

Water levels in Upstream Raise 92 are monitored monthly using 15 piezometers located within the placed CCR slopes of the facility. In addition, two inclinometers were installed in the CCR slopes of the facility to monitor slope movements associated with ongoing consolidation of FGD material in the facility. The plan view location of each piezometer and inclinometer is shown in Figure 4.

Piezometer measurements for the past year are included in Appendix D-1. Piezometers PZ-1 through PZ-12 were constructed near the perimeter bottom ash seepage piping and have historically fluctuated by less than approximately 3 feet as the facility height has increased. Piezometer PZ-12 has been noted to be dry during the second half of 2018, which may be due to FGD material being directed into Upstream Raise 91 during this time period. Piezometers PZ-13 (D and S) and PZ-14 (D and S) were constructed along the side slopes of the facility and show greater variability since installation as they are nearer to the FGD material pool in the center of the facility.

In 2016, piezometer PZ-13D rose approximately 15 feet, and has equilibrated at a new, higher level. Preliminary evaluation of Piezometer PZ-13D suggests that the casing may be damaged and connecting the water within the piezometer to a different zone than the screened portion of the piezometer. This piezometer will continue to be monitored, but may be removed from future monitoring if deemed to be damaged.

Inclinometer measurements taken since 2014 are shown in Appendix D-2. Inclinometer measurements show a general trend that supports the consolidation of the FGD material in the middle of the facility. Consolidation of thicker zones of FGD material in the middle of the facility is greater than consolidation of FGD material zones near the perimeter. Therefore, the inclinometers show a trend of the rigid fly ash and bottom ash material settling toward the center of the facility. Inclinometers appear to show that movement is slowing as a majority of the loading of the facility moves inward, further from the inclinometers.

3.6 Signs of Structural Weakness or Other Observations that Could Affect Stability

No signs of structural weakness or other observations that could affect the stability of Upstream Raise 92 were observed during the site inspection in September 2018.

4.0 SUMMARY AND CONCLUSIONS

An annual visual inspection was performed for Upstream Raise 92 at Coal Creek Station on September 19, 2018. The inspection met the requirements for CCR surface impoundments under 40 CFR Part 257.83. Golder observed good vegetation and site maintenance and did not identify significant structural deficiencies such as significant seepage, settlement, or unexpected cracking during visual observations of Upstream Raise 92. Overall, the facility appeared to be in good condition at the time of the visual evaluation.

In addition to annual inspections by the Professional Engineer, trained and qualified site personnel will continue to perform the required weekly facility inspections to look for signs of potential structural weaknesses. Instrumentation (inclinometers and piezometers) will be monitored regularly to ensure proper operation of the equipment and to evaluate the overall structural stability of the facility.

Minor maintenance items that may need to be continually addressed include monitoring the size and shape of cracks in CCR downstream slope and CCR crest of the facility, repairing large animal burrows as they appear, monitoring erosion and vegetative success of the re-configured surface water channel along the south toe of the facility (including construction to complete downchute channels and hydraulic jump basins prior to final cover of the entirety of Upstream Raise 92), clean-out of collected material in the perimeter channels and maintaining gravity and culvert piping between Upstream Raise 91 and Upstream Raise 92, re-seeding of CCR downstream slopes with final cover, and removal of any woody vegetation growing on the berm downstream slopes. In addition, the inflow and outflow piping should be monitored regularly to ensure proper conveyance of water to and from the facility.

Golder Associates Inc.



Craig Schuettpelez, PE
Senior Project Engineer



Todd Stong, PE
Associate and Senior Consultant

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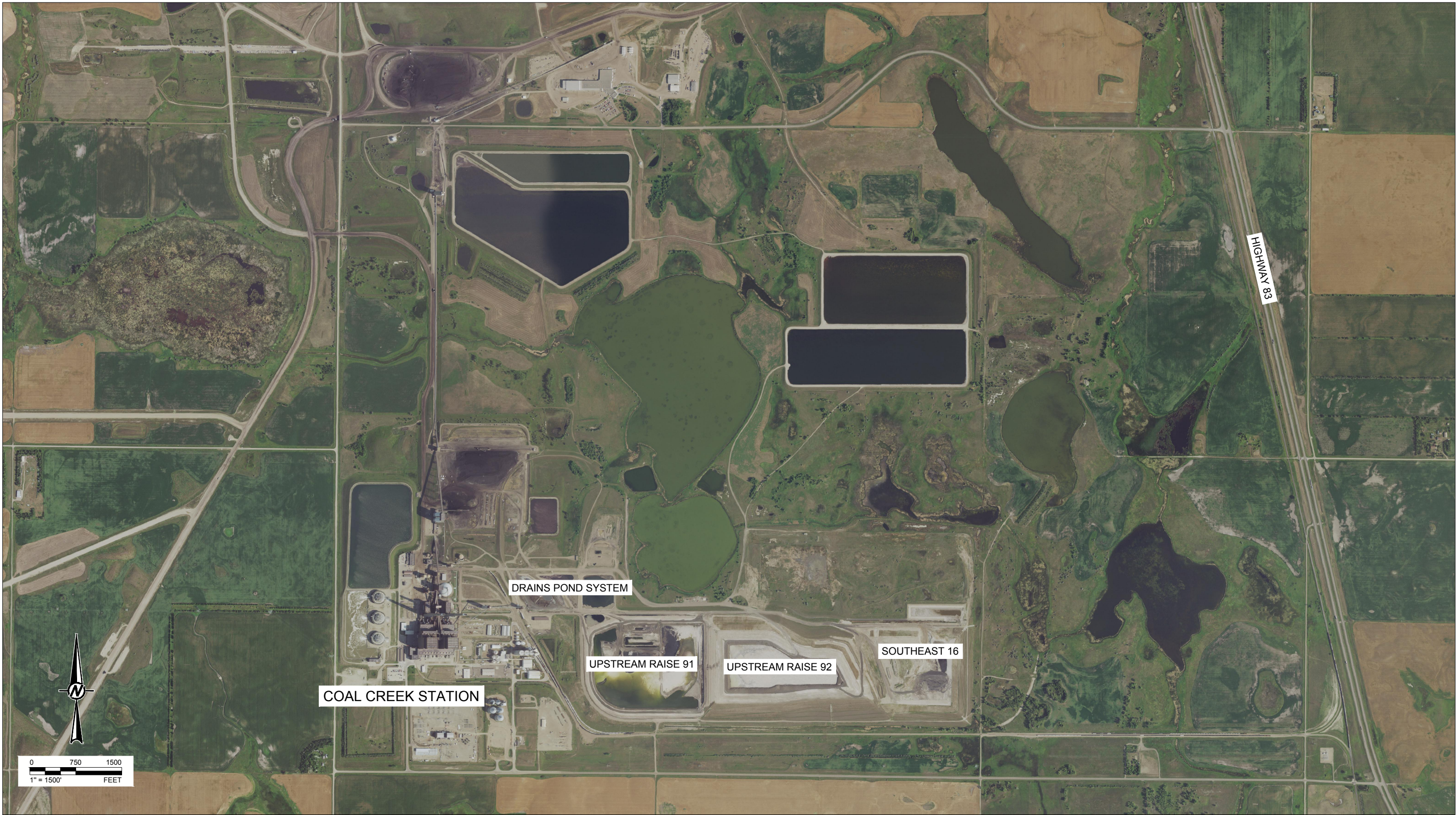
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FIGURES

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- REFERENCES**
1. AERIAL IMAGE FROM UNITED STATES DEPARTMENT OF AGRICULTURE NATIONAL AERIAL IMAGERY PROGRAM, PUBLISHED IN 2018.

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GREAT RIVER ENERGY
COAL CREEK STATION
UNDERWOOD, NORTH DAKOTA

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PROJECT
2018 ANNUAL CCR FACILITY INSPECTION REPORT

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COAL CREEK STATION - SITE OVERVIEW

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REFERENCES

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UNDERWOOD, NORTH DAKOTA

CONSULTANT



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PROJECT
2018 ANNUAL CCR FACILITY INSPECTION REPORT

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UPSTREAM RAISE 92
SITE OVERVIEW

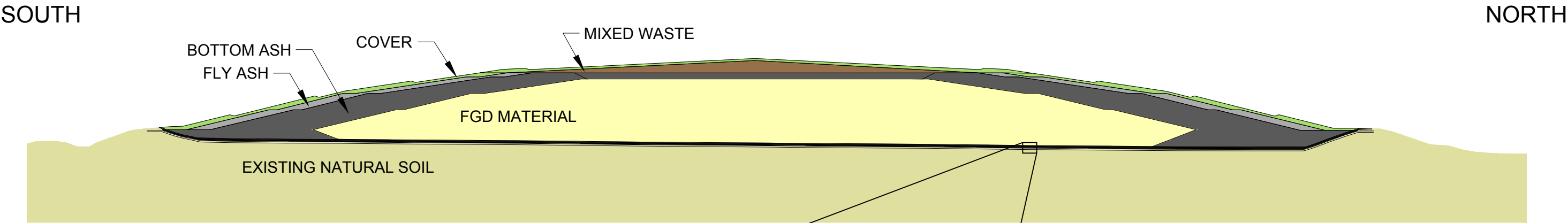
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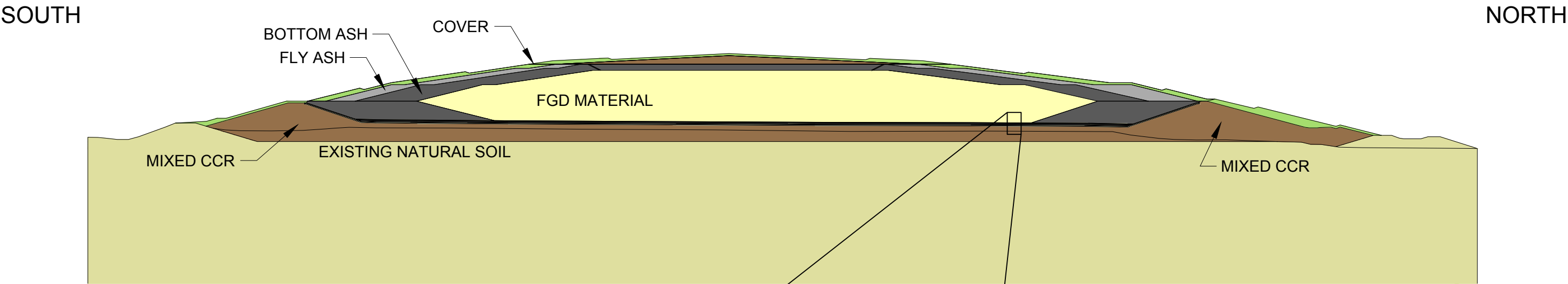
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WEST SIDE



EAST SIDE

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LEGEND

PZ-2 PIEZOMETER

PZ-14 SET OF TWO NESTED PIEZOMETERS

IN-2 INCLINOMETER

- REFERENCES**
1. FOREGROUND AERIAL IMAGE FROM GREAT RIVER ENERGY PHOTOGRAPH TAKEN JUNE 2018.
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2018 ANNUAL INSPECTION REPORT

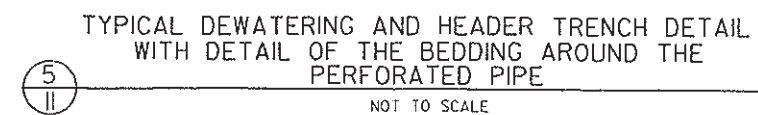
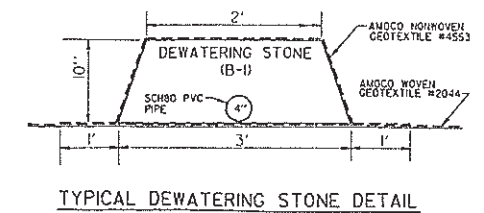
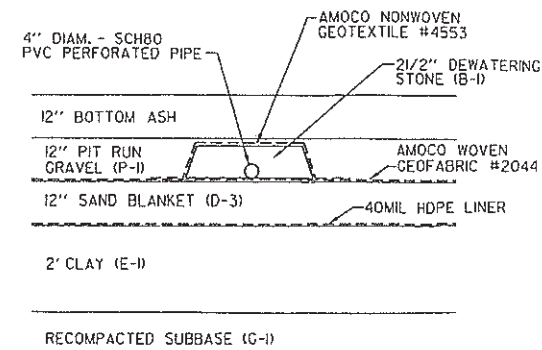
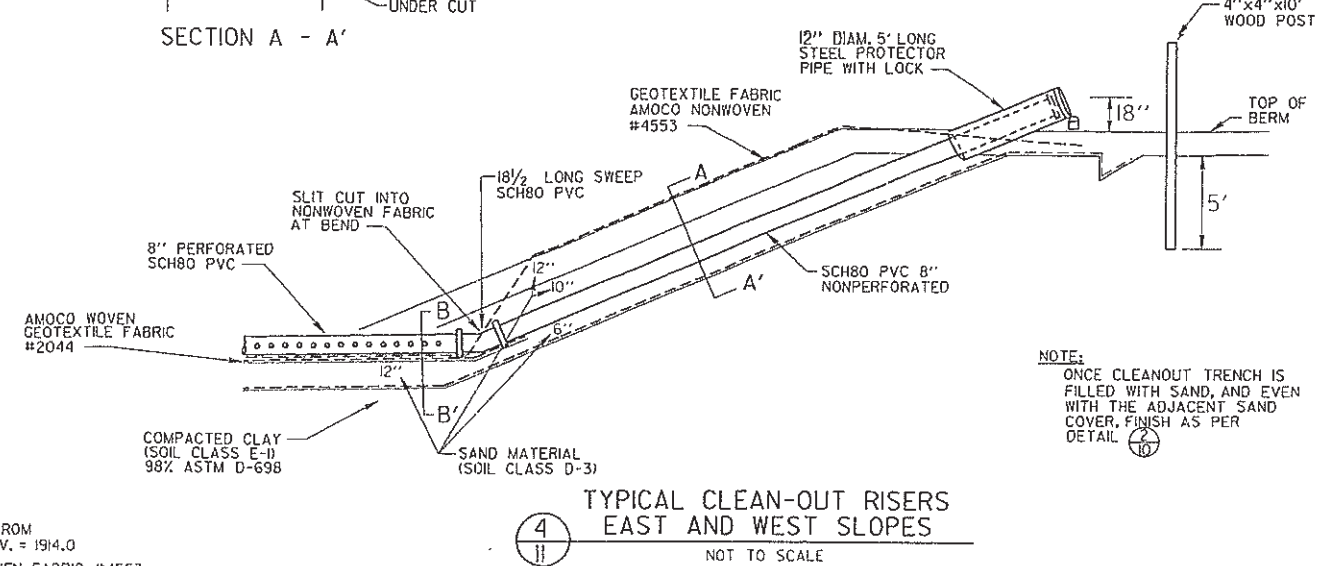
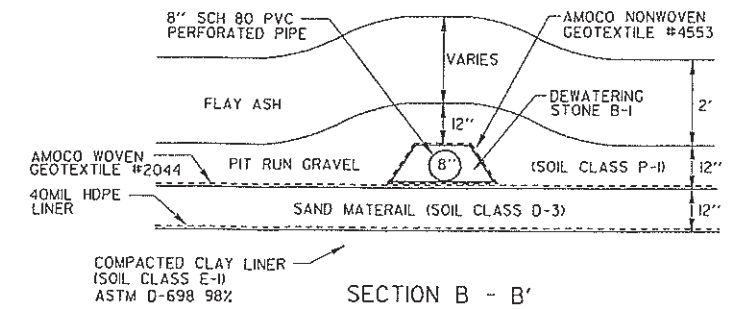
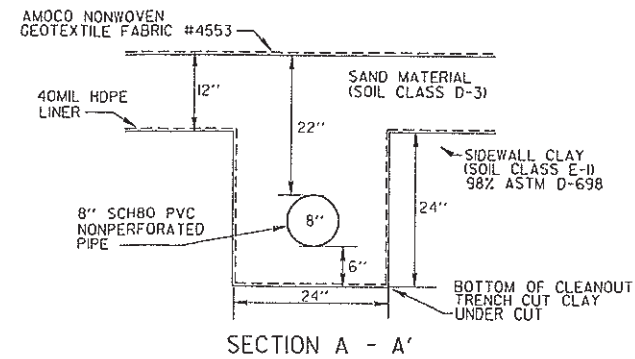
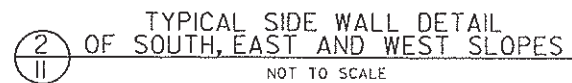
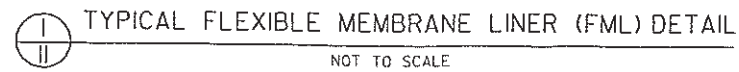
TITLE
UPSTREAM RAISE 92
INSTRUMENTATION OVERVIEW

PROJECT NO. 1893823	REV. B	FIGURE 4
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1 in IF THIS MEASUREMEN DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B

APPENDIX A

Selected Construction Drawings and Permit Drawings

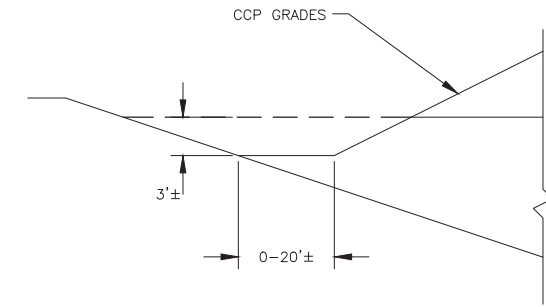


REDUCED
DRAWING
DO NOT
SCALE

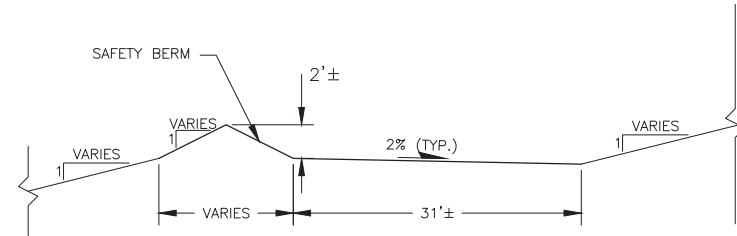
DRAWING NO.	NOT TO SCALE		COOPERATIVE POWER EDEN PRAIRIE, MINN. AND UNITED POWER ELK RIVER, MINN.	Foth & Van Dyke
	SCALE	COAL CREEK STATION EAST HALF OF SOUTH ASH POND		
DETAILS				
REVISIONS / REMARKS NO. BY DATE 1. BY DATE:			APPROVED BY: <i>NJP</i> DATE: 2-6-90	
SURVEY DATA: SURVEYED BY: LS DATE:			RECORDED BY: <i>LS JV</i> DATE: 2-6-90 PREPARED BY: <i>WJA</i> DATE: 2-6-90	
RECORD DRAWINGS OF COMPLETED CONTRACTORS AND/OR OWNERS RECORDS DURING CONSTRUCTION. BY <i>WJA</i> DATE 2-6-90				

REUSE OF DOCUMENTS

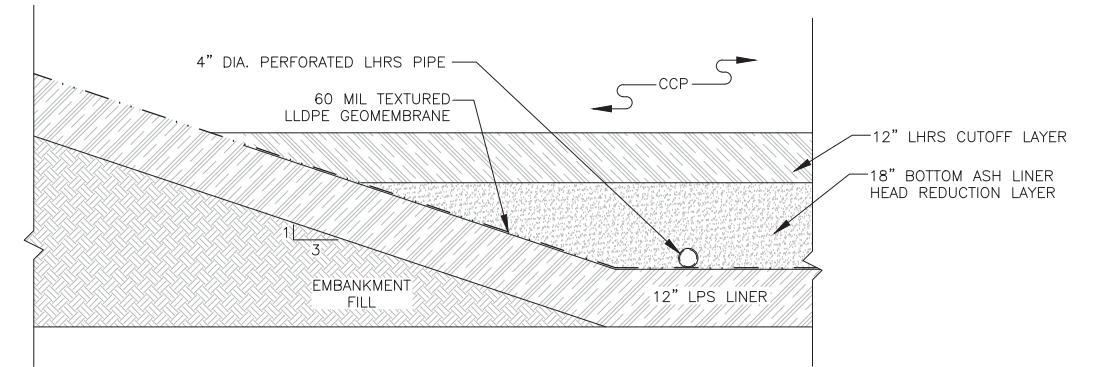
THIS DOCUMENT HAS BEEN DEVELOPED FOR A SPECIFIC APPLICATION AND NOT FOR GENERAL USE. THEREFORE, IT MAY NOT BE USED WITHOUT THE WRITTEN APPROVAL OF FOIH & VAN DYKE AND ASSOCIATES. UNAPPROVED USE IS THE SOLE RESPONSIBILITY OF THE UNAUTHORIZED USER.



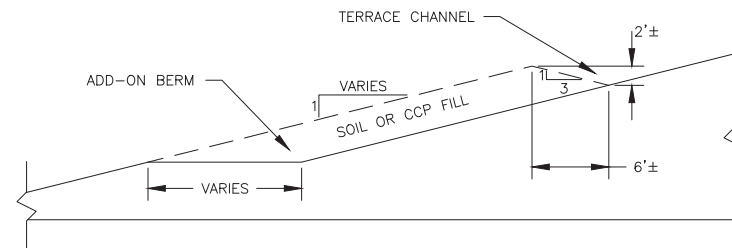
1 CONTACT WATER CONTROL DITCH
11 NOT TO SCALE



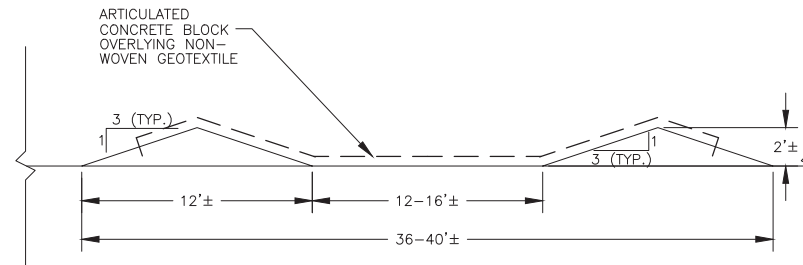
2 ACCESS HAUL ROAD
11 NOT TO SCALE



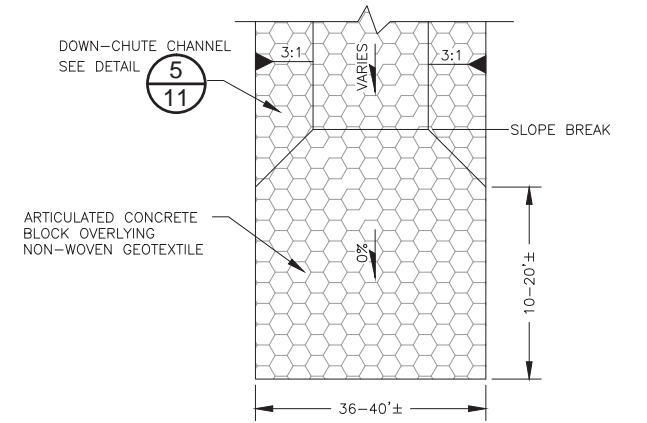
3 SW 16 INTERMEDIATE LINER/CAP AND LHRS DETAIL
11 NOT TO SCALE



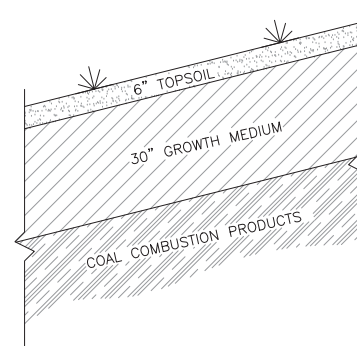
4 TERRACE CHANNEL DETAIL
11 NOT TO SCALE



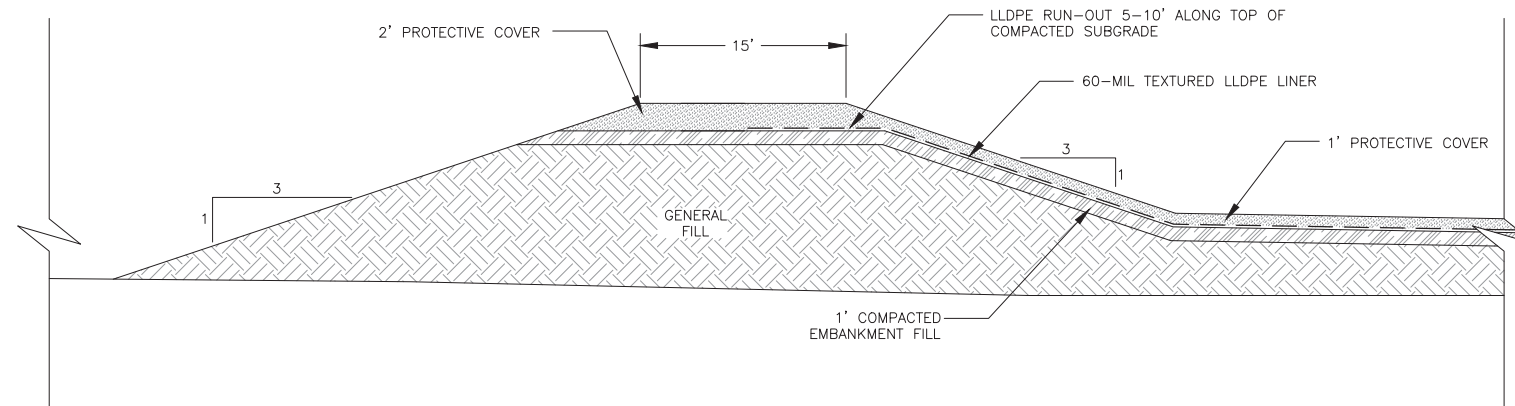
5 TYPICAL DOWN-CHUTE CHANNEL
11 NOT TO SCALE



6 HYDRAULIC JUMP BASIN
11 NOT TO SCALE





7 FINAL COVER DETAIL
11 NOT TO SCALE



8 SUMP EXTENSION DETAIL
11 NOT TO SCALE

DRAFT

PROJECT	GREAT RIVER ENERGY COAL CREEK STATION PERMIT NO. SP-033 PERMIT MODIFICATION		
TITLE	DETAIL SHEET 2		
FILE No.	11381519A012		
PROJECT No.	113-81519		
			
		11	

ENGINEER'S STAMP	NO.	REVISION DESCRIPTION	DATE	DESIGN	CADD	CHECK	REVIEW
ORIGINAL DRAWING STAMPED BY TODD STONG, REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF NORTH DAKOTA (PE #6144), IS ON FILE AT GOLDER ASSOCIATES' LAKEWOOD, COLORADO, OFFICE.	△						
	△						
	△						
	△						
	△						
	△	ISSUED FOR PERMIT MODIFICATION	11/30/12	CCS	CCS	TJS	RRJ
	△	ISSUED FOR CLIENT REVIEW	10/24/12	CCS	CCS	TJS	RRJ

APPENDIX B

Visual Observations Checklist

IMPOUNDMENT INSPECTION CHECKLIST

Facility Name: Upstream Raise 92

Owner and Address: Great River Energy – Coal Creek Station

Purpose of Facility: CCR Containment

Legal: Section 16

Township: 145N

Range: 82W

County: McLean

Inspected By: Todd Stong/Craig Schuettpehlz/Paul Schlicht

Inspection Date: September 19, 2018

Weather: Sunny, 45°F - 65°F, No Precipitation

ITEM	Y	N	N/A	REMARKS
1. Water Levels				
a. High water mark			X	
b. Current water level			X	Insignificant water observed
2. Inflow Structure				Flue gas desulfurization piping (not currently being used, deposition in Upstream Raise 91)
a. Settlement		X		
b. Cracking		X		
c. Corrosion		X		
d. Obstacles in inlet		X		
e. Riprap/erosion control		X		
3. Outflow Structure				Drains to Upstream Raise 91
a. Settlement		X		
b. Cracking		X		
c. Corrosion		X		
d. Obstacles in outlet		X		
e. Riprap/erosion control		X		
4. CCR Placement Areas				
a. CCR Upstream Slope erosion		X		
b. CCR Upstream Slope cracks/settlement		X		
c. CCR Crest exposed to heavy traffic	X			Cat 777
d. CCR Crest damage from vehicles/machinery		X		
e. CCR Crest cracks/settlement	X			Settlement of fly ash over FGD material
f. CCR Downstream slope erosion	X			Minor erosion of fly ash
g. CCR Downstream slope cracks/settlement	X			Settlement of fly ash over FGD material
5. Covered Downstream Slopes				
a. Downstream Slope erosion	X			Minor erosion of temporary cover
b. Downstream Slope rodent burrows	X			South side
c. Downstream Slope vegetation	X			Less vegetation above 1950 feet and on temporary cover, healthy vegetation below 1950 feet
d. Downstream Slope seepage/sloughs/cracks/settlement	X			Cracks due to settlement of fly ash over FGD material at 1974 feet (no cracks noted below 1974 feet on covered slopes)
6. Perimeter Berm				
a. Upstream Slope erosion (exposed liner)		X		
b. Upstream Slope rodent burrows		X		
c. Upstream Slope vegetation		X		
d. Upstream Slope cracks/settlement		X		
e. Upstream Slope riprap/other erosion protection	X			Fly ash protective cover in perimeter ditches
f. Crest exposed to heavy traffic	X			North side haul road (Cat 777)
g. Crest damage from vehicles/machinery		X		
h. Crest comparable to design width	X			
i. Crest rodent burrows		X		
j. Downstream Slope erosion	X			Minor erosion near toe of slope
k. Downstream Slope rodent burrows	X			Mostly small burrows on south side
l. Downstream Slope vegetation	X			Healthy grass and reeds, some sparse vegetation near toe of slope
m. Downstream Slope seepage/sloughs/cracks/settlement		X		Minor over-steepening of slope near toe as a result of re-grading of the south drainage channel
7. Toe				
a. Vegetation	X			Healthy grass and reeds
b. Rodent burrows	X			Mostly small burrows on south side
c. Seepage/sloughs/cracks/settlement		X		
d. Drainage conditions	X			Some standing water

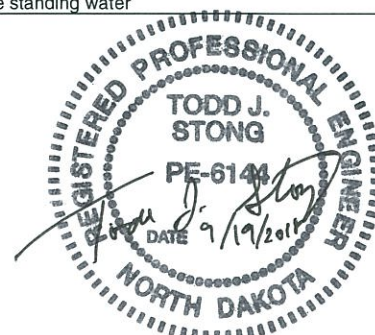
General Remarks: No significant issues. Cracking of fly ash within the facility is to be expected and should be monitored. Minor maintenance includes addressing small burrows, maintaining fly ash protective cover, maintaining inflow and outflow piping, and addressing erosion as observed.

Name of Engineer: Todd Stong

Date: 9/19/2018

Engineering Firm: Golder Associates Inc.

Signature: *Todd Stong*



PROFESSIONAL ENGINEER SEAL

APPENDIX C

Photographs

Path: U:\dewar\great_river_energy\COAL_CREEK\09_PROJECT\1893823\Annual_Inspection\Photology | File Name: 2018 Annual Inspection_Photology.dwg | Last Edited By: cshuettpeltz | Date: 2019-01-03 | Time: 3:41:54 PM | Printed By: CShuettpeltz | Date: 2019-01-03 | Time: 3:44:57 PM



LEGEND

PHOTOGRAPH NUMBER AND DIRECTION

- REFERENCES**
1. FOREGROUND AERIAL IMAGE FROM GREAT RIVER ENERGY PHOTOGRAPH TAKEN JUNE 2018.
 2. BACKGROUND AERIAL IMAGE FROM UNITED STATES DEPARTMENT OF AGRICULTURE NATIONAL AERIAL IMAGERY PROGRAM, PUBLISHED IN 2018.

CLIENT
GREAT RIVER ENERGY
COAL CREEK STATION
UNDERWOOD, NORTH DAKOTA

CONSULTANT	YYYY-MM-DD	2019-01-03
DESIGNED	KAC	
PREPARED	KAC	
REVIEWED	CCS	
APPROVED	TJS	



PROJECT
2018 ANNUAL CCR FACILITY INSPECTION REPORT

TITLE
UPSTREAM RAISE 92
PHOTOGRAPH LOCATIONS

PROJECT NO.
1893823

REV.
B

FIGURE
1

1 in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM ANSI B

Upstream Raise 92



Photograph 1 (West CCR crest)
Bottom ash CCR crest (IMGP6384.JPG)



Photograph 2 (West CCR upstream slope)
Gravity drain outlet pipes (IMGP6390.JPG)

Upstream Raise 92



Photograph 3 (West CCR downstream slope)
Crack and erosion in fly ash CCR downstream slope (IMGP6392.JPG)



Photograph 4 (South CCR downstream slope)
Crack in fly ash bench (within the contact water separation channel) (IMGP6395.JPG)

Upstream Raise 92



Photograph 5 (South CCR covered downstream slope)
Cracking of the final cover slope near the bench at elevation 1974 feet (IMGP6397.JPG)



Photograph 6 (South CCR covered downstream slope)
South CCR downstream slope to final cover downstream slope (IMGP6404.JPG)

Upstream Raise 92



Photograph 7 (East CCR upstream slope)
Flue Gas Desulfurization (FGD) pool and bottom ash upstream CCR slopes (IMGP6407.JPG)



Photograph 8 (North CCR downstream slope)
Minor erosion of fly ash on CCR downstream slope (IMGP6424.JPG)

Upstream Raise 92



Photograph 9 (North CCR downstream slope)
Crack in fly ash near piezometers (IMG6425.JPG)



Photograph 10 (North CCR downstream slope)
Crack in fly ash CCR crest (approximately 10 inches wide) (IMG6427.JPG)

Upstream Raise 92



Photograph 11 (North CCR downstream slope)
Sump riser on east half of Upstream Raise 92 (IMGP6429.JPG)



Photograph 12 (West CCR downstream slope)
Crack on CCR downstream slope bench at elevation 1974 feet (approximately 6 inches wide, 6 feet deep, 6 inches offset vertically) (6ccs.JPG)

Upstream Raise 92



Photograph 13 (West CCR downstream slope)

Crack on CCR downstream slope bench at approximate elevation 1970 feet (approximately 4 inches wide, 3 feet deep, 4 inches offset vertically) (13ccs.JPG)



Photograph 14 (South downstream slope)

Over-steepened toe (remnant of construction, no seepage present) (23ccs.JPG)

Upstream Raise 92



Photograph 15 (South berm downstream slope)
Typical berm downstream slope (26ccs.JPG)



Photograph 16 (East CCR downstream slope)
Contact water ditch at elevation 1950 feet on east CCR downstream slope (48ccs.JPG)

Upstream Raise 92



Photograph 17 (North downstream slope and haul road)
Contact water channel and recently installed culvert (53ccs.JPG)



Photograph 18 (North downstream slope)
Mossy vegetation in grass (no seepage) (56ccs.JPG)

Upstream Raise 92



Photograph 19 (North berm downstream slope and haul road)
Steep downstream slope below recently re-constructed haul road ramp (66ccs.JPG)



Photograph 20 (North berm downstream slope)
Typical berm downstream slope (69ccs.JPG)

Upstream Raise 92



Photograph 21 (SW CCR covered downstream slope)
Articulated Concrete Block (ACB) downchute, thick vegetation between blocks along the slope (1PDS.JPG)



Photograph 22 (South CCR downstream slope)
Animal burrows near the perimeter road on the CCR downstream slope (9PDS.JPG)

Upstream Raise 92



Photograph 23 (South CCR covered downstream slope)
Articulated Concrete Block (ACB) downchute on covered CCR downstream slope (19PDS.JPG)



Photograph 24 (North contact water ditch and haul road)
Recently installed concrete culvert under north haul road (25PDS.JPG)

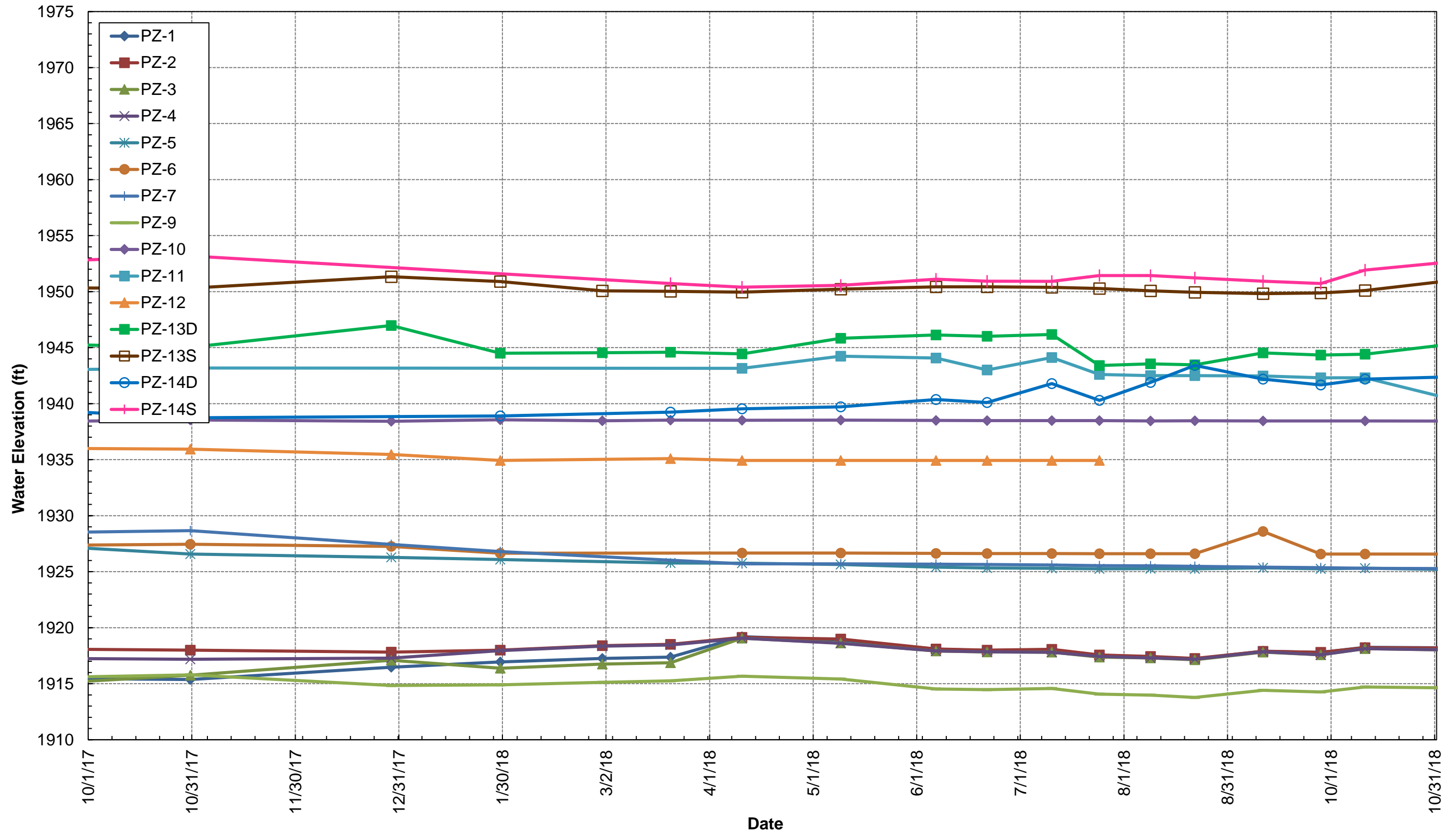
APPENDIX D

Instrumentation Results

APPENDIX D-1

Piezometer Information

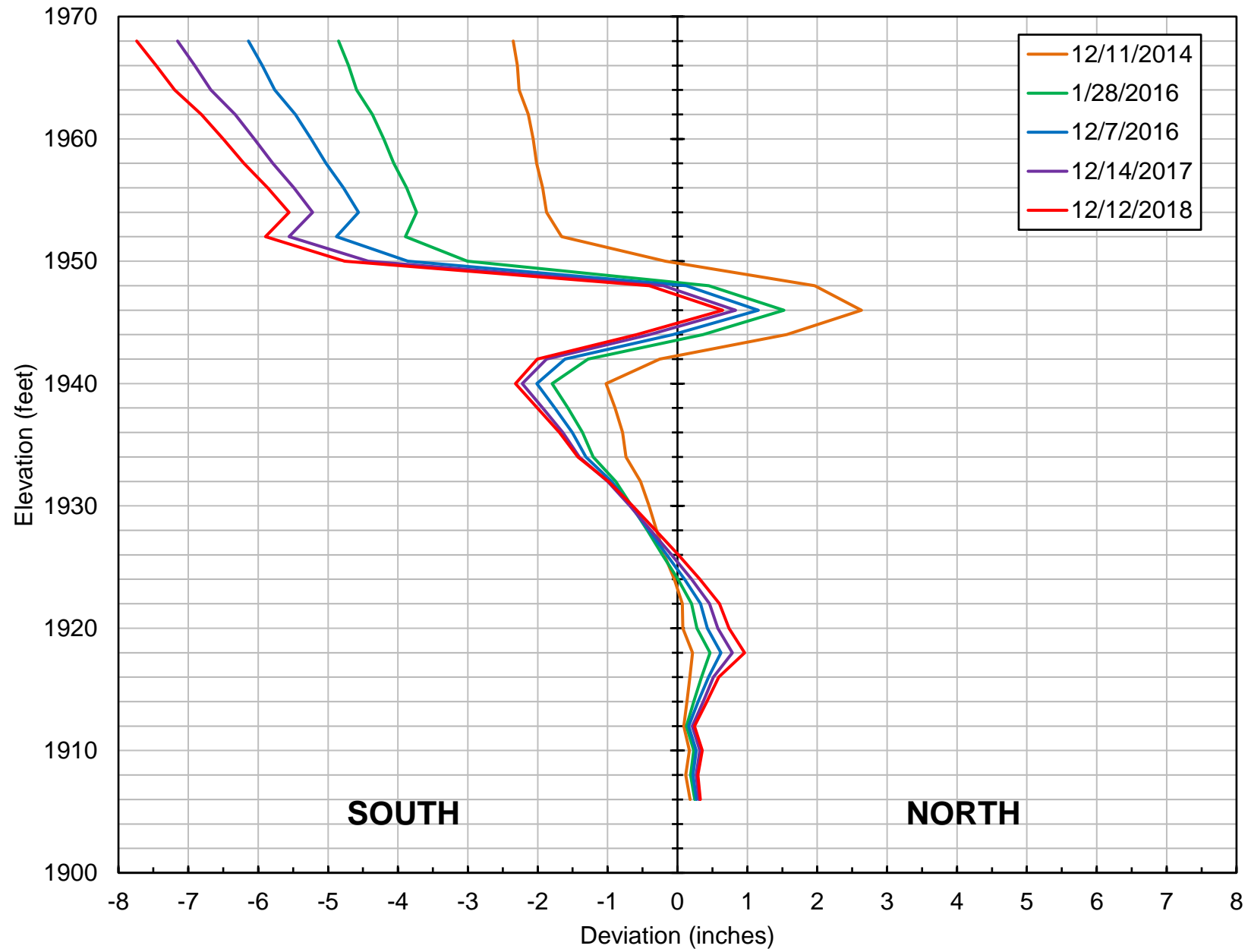
Upstream Raise 92 Piezometer Elevations



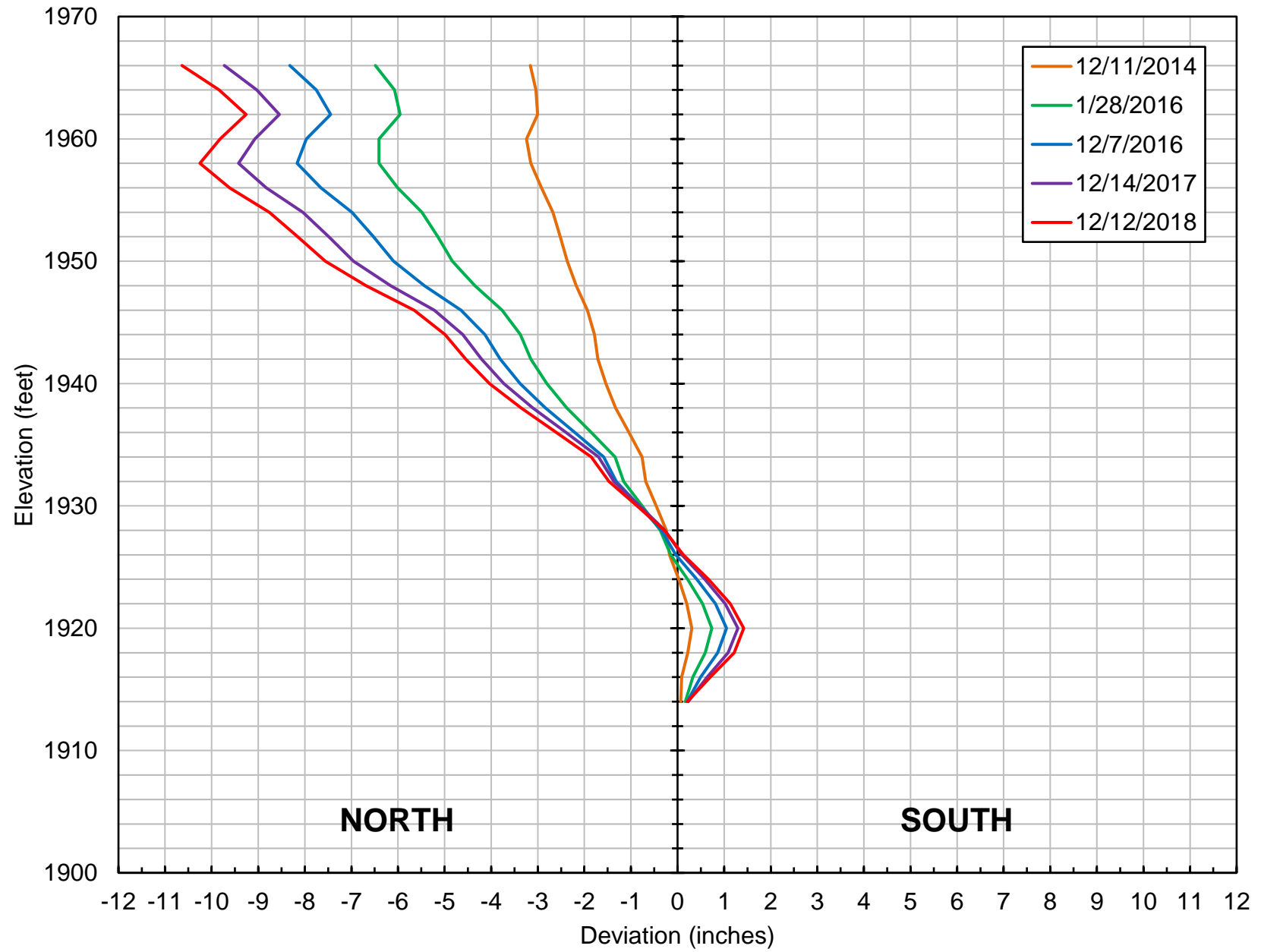
APPENDIX D-2

Inclinometer Information

Upstream Raise 92
Inclinometer IN-3 Summary



Upstream Raise 92
Inclinometer IN-4 Summary





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